	General Notes		Steel Grades
G01	The Design and Details shown in these Drawings are applicable to this Project only.	300 PLUS	Universal Beams & Columns, Parallel Flange Channels, Large Angles to AS/NZS3679.1
G02	These Drawings shall be read in conjunction with all Architectural Drawings, other Consultants' Drawings, Specifications and such other Written	250	Flats, Small Angles, Taper Flange Beams & Columns to AS/NZS3679.1
	Instructions as may be issued during the course of the Project. Any discrepancy shall be referred to the Engineer before proceeding with the work.	300	Welded Sections to AS/NZS3679.2
G03	All Materials and Workmanship shall be in accordance with the relevant and	250	Hot Rolled Plates, Floor Plates & Slabs to AS/NZS3678
G03	current SAA Codes and Authorities Except where varied by the Project Specifications.	250	Hollow Sections to AS1163. Circular Sections less than 165mm Outside Diameter. Sections other than the above
G04	The Structure must be maintained in a stable condition and no part must be overloaded during construction. Temporary bracing must be designed and	G450-G550	Cold Form AS4600. Unless noted otherwise, all purlins, plates & brackets are G450.
	provided by the contractor(s) to keep the building works and excavations	G500	Slab mesh and deformed reinforcement bars.
	stable at all times.	G2070	AS2841. Galvanized Steel Wire Strand. Unless noted otherwise,
G05	The Issuer of these Designs reserves the right to alter Specifications and Designs as it may see fit without prior notification or penalty.		all Cable Bracing is G2070.
G06	The Structure has not been designed for snow loads.	Australian	Standard <sup>™</sup> & Australian/New Zealand Standard <sup>™</sup> Adopted
G07	Connections may require on-site drilling by the contractor(s).	AS 1111.1	ISO Metric Hexagon Bolts and Screws – Product Grade C Part 1: Bolts
	Foundation Notes	AS/NZS 1163	Cold-formed Structural Steel Hollow Sections
F01	Foundations have been designed assuming an allowable bearing capacity of	AS/NZS 1170.0	Structural Design Actions - Part 0: General Principles
	100 kPa.	AS/NZS 1170.1	Structural Design Actions - Part 1: Permanent, Imposed & Other
F02	The Owner is advised to obtain a Geotechnical Report and/or Site Foundation Report to confirm F01 above.		Actions
F03	Footings shall be located centrally under walls and columns unless noted	AS/NZS 1170.2	Structural Design Actions - Part 2: Structural Design, Wind Action
1 03	otherwise.	AS/NZS 1170.4	Structural Design Actions - Part 4: Earthquake Actions in Australia
F04	All Footings to be founded a minimum of 400 mm into Natural Ground. Do not	AS 1252	Structural Assemblies
	Found Footings in uncontrolled fill.	AS/NZS 1554	Structural Steel Welding
F05	Engineer to be contacted if Foundation Conditions vary from F01 above.	AS 4100	Steel Structures
	Steelwork Notes	AS/NZS 4600	Cold-formed Steel Structures
004	All Workmanship and Materials shall be in accordance with AS 4100, AS/NZ	AS/NZS 4671	Steel Reinforcing Materials
S01		AS/NZS 4680	Hot-dip Galvanized (Zinc) Coatings on Fabricated Ferrous Articles

S01	All Workmanship and Materials shall be in accordance with AS 4100, AS/NZ
	4600 and AS 1554 except where varied by the Project documentation.

- S02 Bolting Categories are identified on the Structural Drawings as follows: 4.6/S Commercial Bolts of Grade 4.6 to AS 1111 Snug Tightened 8.8/S High Strength Structural Bolts of Grade 8.8 to AS 1252 Snug Tightened
- S03 Unless noted otherwise, all M16 fasteners shall be Category 8.8/S. Standard Fixings for C100, C150, C200 and C250 Sections are M12 bolts. Standard Fixings for C300 and C350 Sections are M16 bolts. No Connection shall have less than 2 bolts.
- S04 All metal cladding should comply with AS/NZS 4680 G550, AZ150 (550 MPa minimum yield stress, 150 g/m2 minimum coating mass).
- S05 Steelwork intended to be concrete encased shall be unpainted. Encasing concrete shall be Grade N25 unless noted otherwise providing a cover adequate to suit fire rating or exposure conditions. Concrete Encasement shall | Foundations have been designed assuming an allowable bearing capacity of 100 be centrally reinforced with 5 mm wire to AS 4671 or 6 mm Structural Grade Bars to AS 4671 at 150 mm pitch.
- S06 Unless noted otherwise, all steel shall con Standard.

G450-G550	Cold Form AS4600. Unless noted otherwise, all purlins, plates & brackets are G450.
G500	Slab mesh and deformed reinforcement bars.
G2070	AS2841. Galvanized Steel Wire Strand. Unless noted otherwise, all Cable Bracing is G2070.
Australian	Standard <sup>™</sup> & Australian/New Zealand Standard <sup>™</sup> Adopted
AS 1111.1	ISO Metric Hexagon Bolts and Screws – Product Grade C Part 1: Bolts
AS/NZS 1163	Cold-formed Structural Steel Hollow Sections
AS/NZS 1170.0	Structural Design Actions - Part 0: General Principles
AS/NZS 1170.1	Structural Design Actions - Part 1: Permanent, Imposed & Other Actions
AS/NZS 1170.2	Structural Design Actions - Part 2: Structural Design, Wind Actions
AS/NZS 1170.4	Structural Design Actions - Part 4: Earthquake Actions in Australia
AS 1252	Structural Assemblies
AS/NZS 1554	Structural Steel Welding
AS 4100	Steel Structures
AS/NZS 4600	Cold-formed Steel Structures
AS/NZS 4671	Steel Reinforcing Materials
AS/NZS 4680	Hot-dip Galvanized (Zinc) Coatings on Fabricated Ferrous Articles
	Region & Site Details
his is a freestand	ding roof only structure. It is assumed that cars, goods or materials

stored under the roof block less than 50 percentage of the cross section exposed to

Suitable for Regions A1-5 & B1. TC2.5 without Shielding: Terrain with some trees or isolated obstructions, terrain in developing outer urban areas with scattered houses, or large acreage developments with more than two and less than 10 buildings per hectare. The site is not on the slope or crest of a hill, ridge or escarpment.

 $V_{design} = V_R \times M_c \times M_d \times (M_{z,Cat} \times M_s \times M_t)$ 

 $V_{design} = 57 \times 1 \times 0.95 \times (0.87 \times 1 \times 1) = 47.11 \text{ m/s}$ 

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	urb NT 0820	2022-09-28	4	A4	NTS	1 of 27	
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Example Customer		dm3 Solutions		Powered by dm3So		utions.com	
JENT:		DRAWN BY:	otherwise, dimensions are in millimetres & drawings are not to so			drawings are not to scale	
Example 7000 x 12000 x 2400 Region B1 Hip Carport		6461	Confidential and the Sole Property of dm3 Solutions. Reproduction in pa or in whole is Prohibited without written approval. Unless specified				
PROJECT:		PROJECT NUMBER:			Solutions. The Information contained herein is Proprietary,		
E JE	xample 7000 x 1200 ENT: xample Customer	xample 7000 x 12000 x 2400 Region B1 Hip Carport  ENT: xample Customer	xample 7000 x 12000 x 2400 Region B1 Hip Carport  ENT: xample Customer  6461  DRAWN BY: dm3 Solutions	xample 7000 x 12000 x 2400 Region B1 Hip Carport  ENT: xample Customer  6461  Confidentia or in wh otherwise dm3 Solutions	xample 7000 x 12000 x 2400 Region B1 Hip Carport  ENT: xample Customer  6461  Confidential and the Sole P or in whole is Prohibited otherwise, dimensions ar dm3 Solutions  Powered	xample 7000 x 12000 x 2400 Region B1 Hip Carport  ENT:  DRAWN BY:  dm3 Solutions  Confidential and the Sole Property of dm3 So or in whole is Prohibited without written as otherwise, dimensions are in millimetres & Powered by dm3Solutions	

**Design Notes & Structural Members** 

7000 x 12000 x 2400

See Rafter Layout

TH22x0.40 G550

TH6110 G550

Cast In 3.0 mm G450

Page 26

See Post & Footing Layout

See Fascia Beam Layout

47.11 m/s

Dimensions **Building Class** 

Columns C

Rafters R

Purlins P

Bridging Footings

All Brackets

Technical Data

Fascia Beam FB

Design Wind Speed

12000 Frame Width **Front Elevation** 

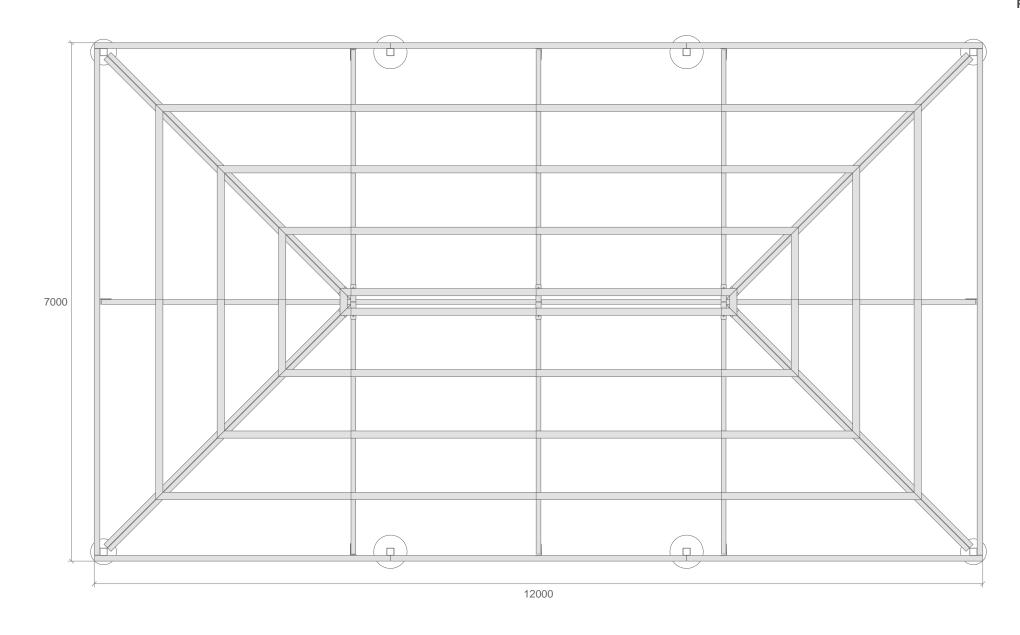
Note: Roof structure is light foot trafficable for maintenance purposes.



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HC4.1.1

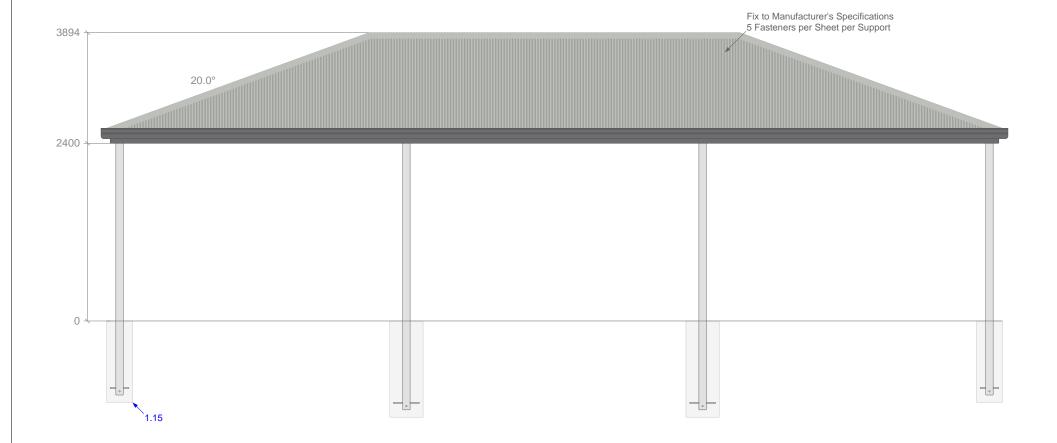
# Plan View



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#### Front View





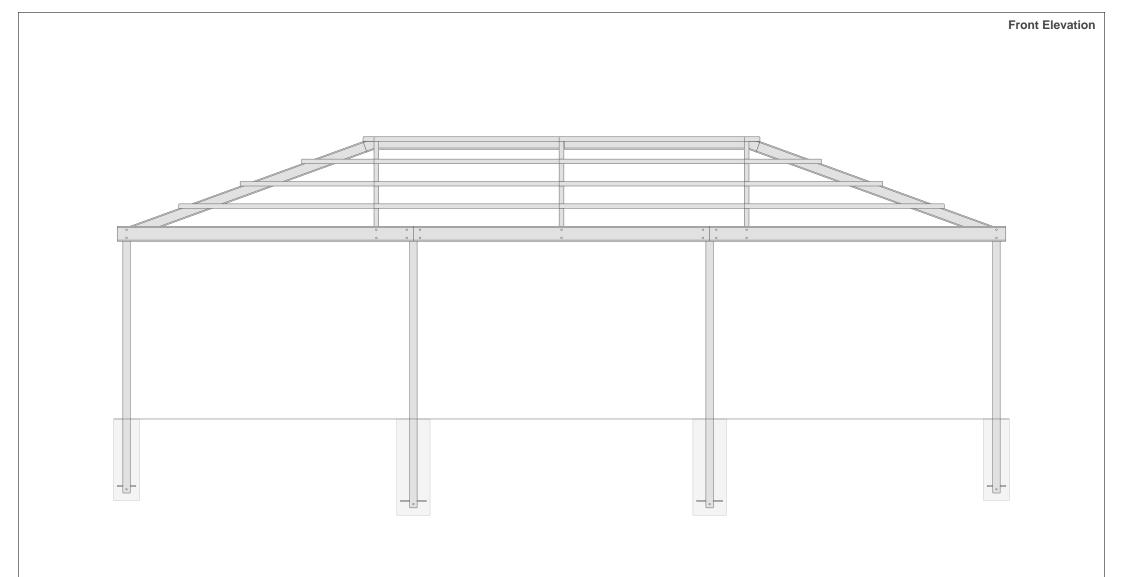
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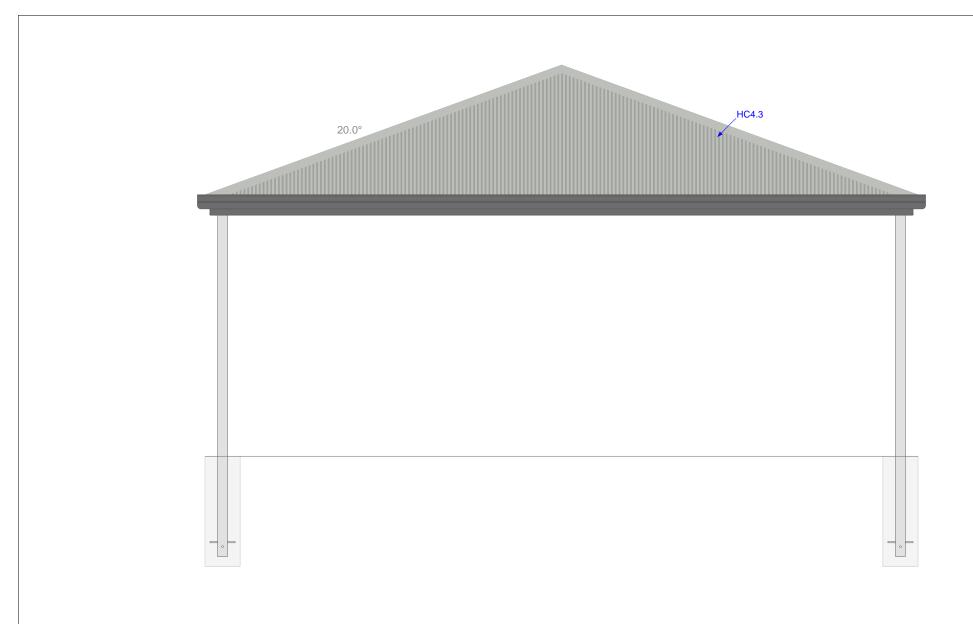
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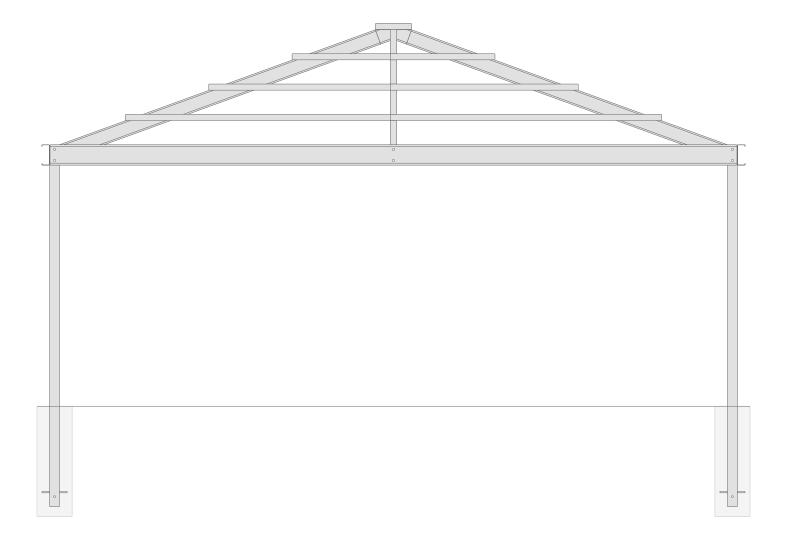


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Left View





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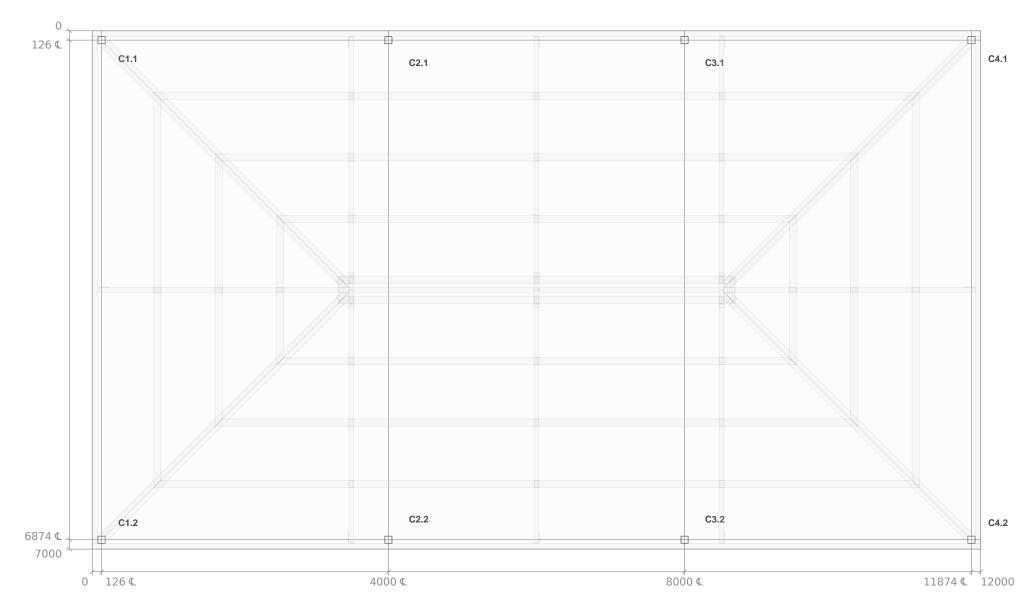
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# **Post & Footing Layout**





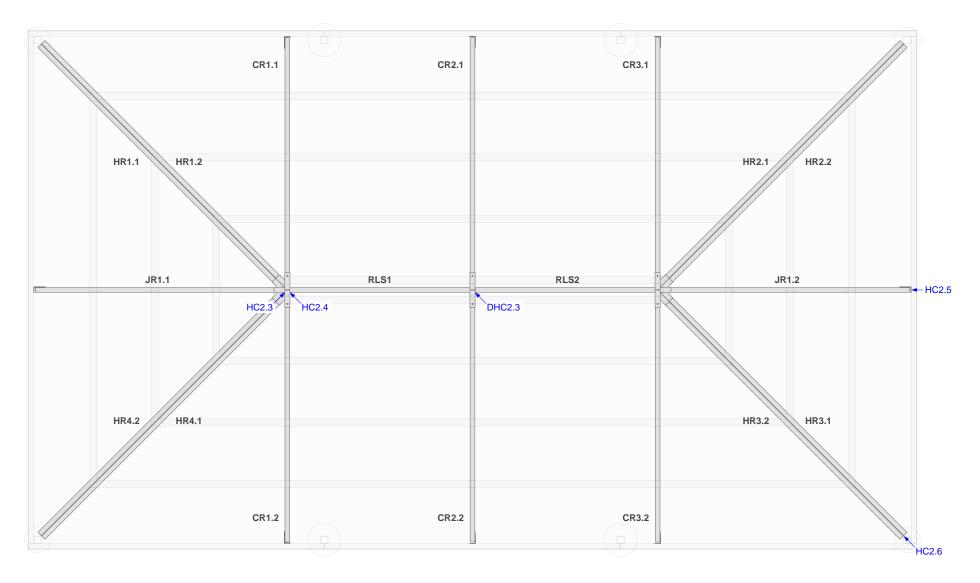
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Example 7000 x 12000 x 2400 Region B1 Hip Carport	6461
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# Rafter Layout



CR = Common Rafter · C15024

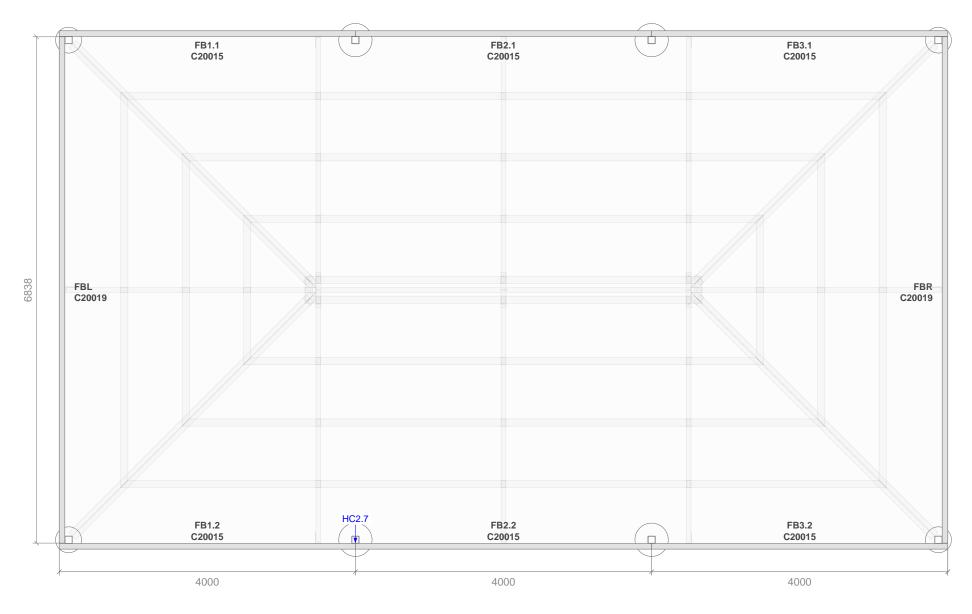
JR = Jack Rafter · C15012 RLS = Ridge Line Strut · C15024 Note: All Rafter to Fascia Connections Use Detail HC2.5 Above

HR = Hip Rafter · C15012



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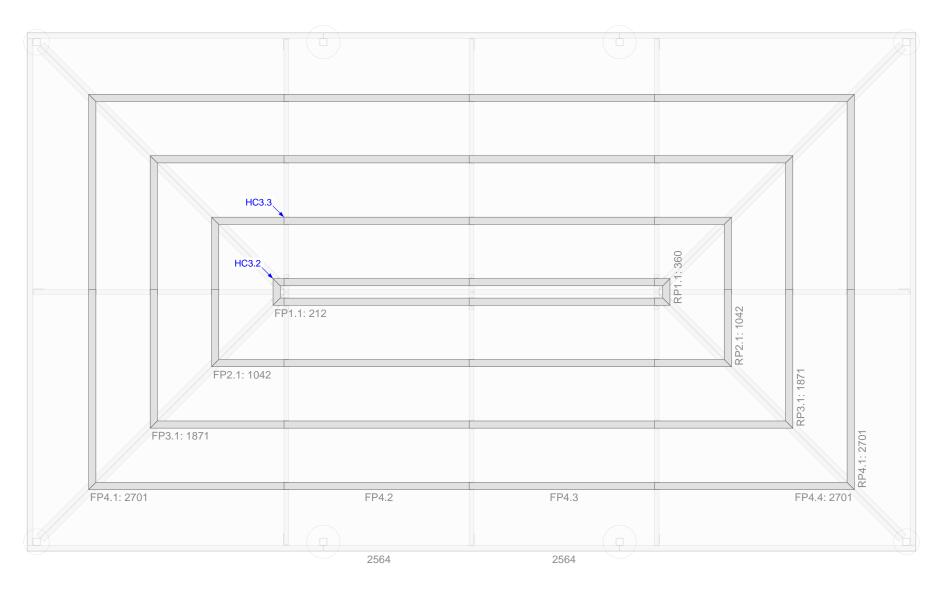
# **Fascia Layout**





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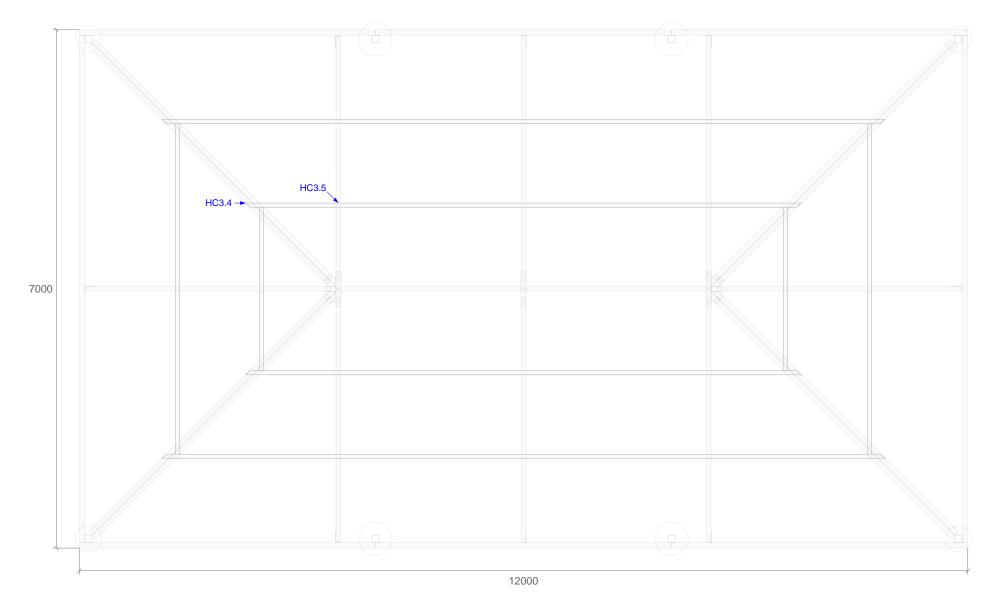
# **Purlin Layout**



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# **Bridging Layout**



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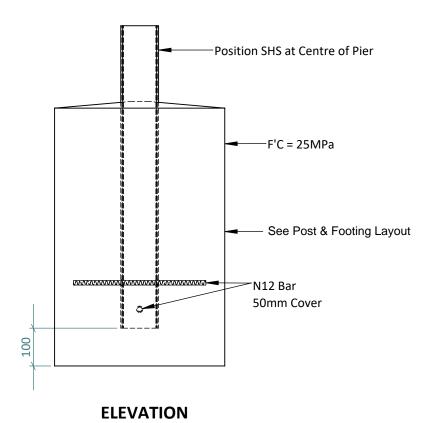
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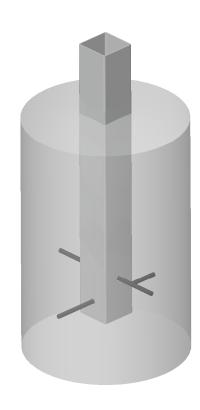
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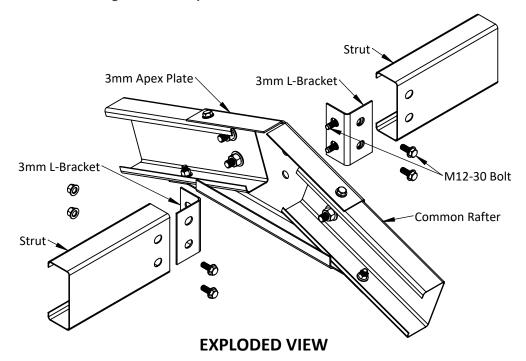


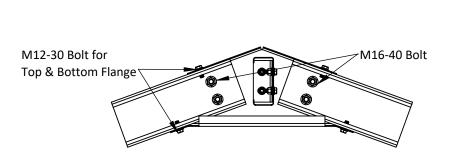
**ISOMETRIC** 

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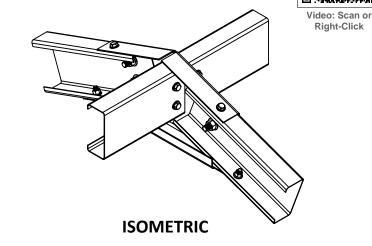
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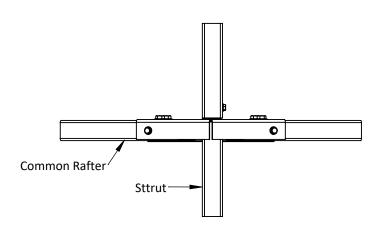
# **《 Detail DHC2.3: Ridge Line Strut Apex Connection Detail**





**ELEVATION** 



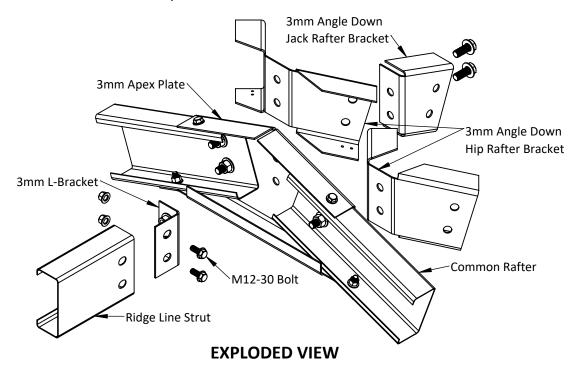


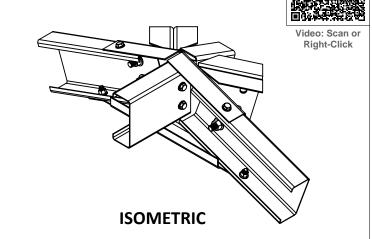
**PLAN** 

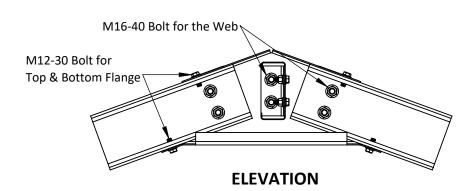


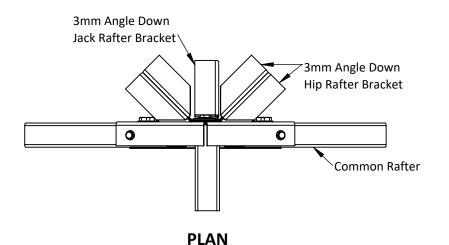
PROJECT: Example 7000 x 12000 x 2400 Region B1 Hip Carport	PROJECT NUMBER: 6461	© 2022 dm3 Solutions. The Information contained herein is Proprietar Confidential and the Sole Property of dm3 Solutions. Reproduction in p or in whole is Prohibited without written approval. Unless specified otherwise, dimensions are in millimetres & drawings are not to scale.  Powered by dm3Solutions.com			
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# **《 Detail HC2.3: Common Rafter Apex Connection Detail**





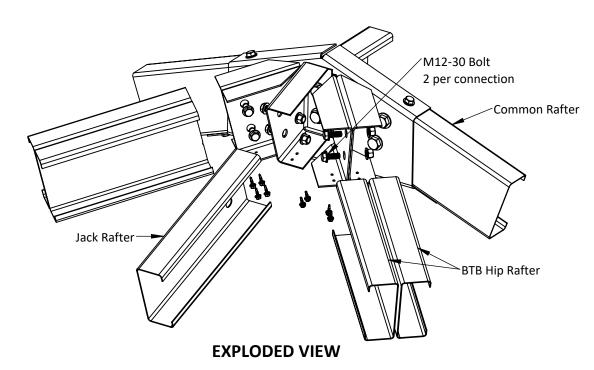


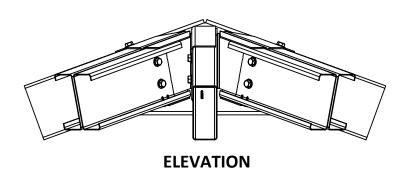


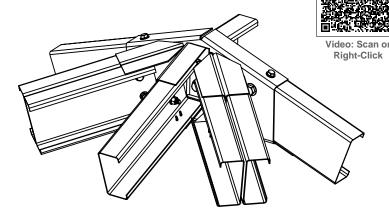
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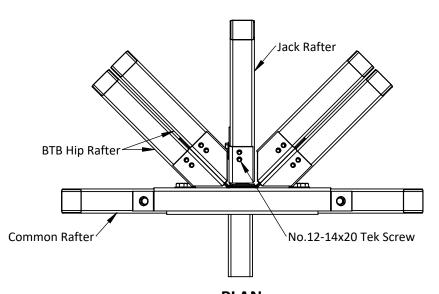
### **« Detail HC2.4: Jack Rafter Apex Connection Detail**







# **ISOMETRIC**



# PLAN (View from underneath of the frames)

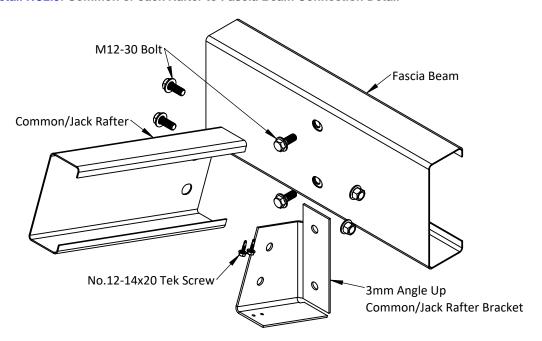


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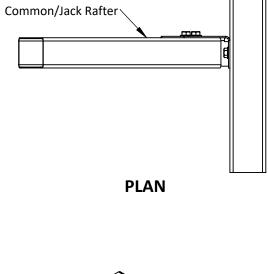
### **《 Detail HC2.5: Common or Jack Rafter to Fascia Beam Connection Detail**



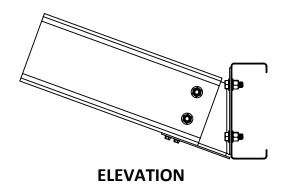
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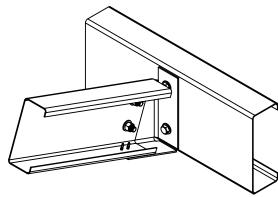




Fascia Beam



Example Street, Suburb NT 0820



**ISOMETRIC** 

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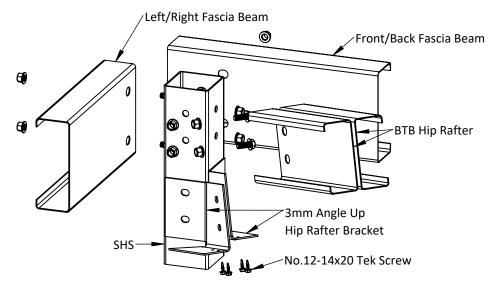
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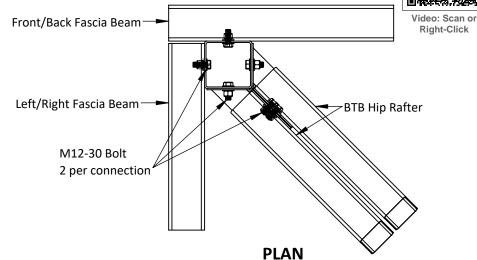
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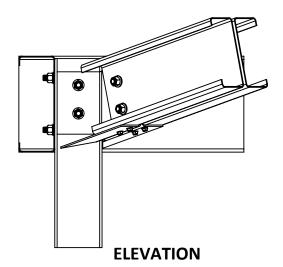
### **« Detail HC2.6: Hip Rafter to Post Connection Detail**

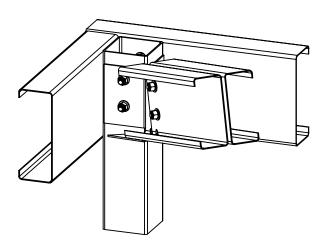






# **EXPLODED VIEW**





# **ISOMETRIC**



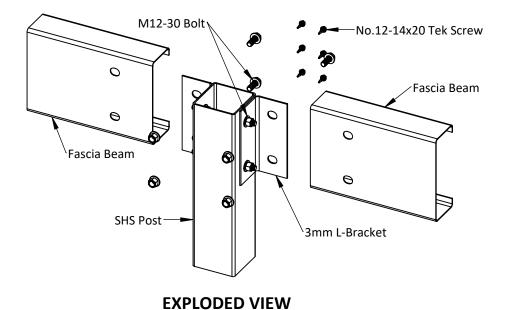
PROJECT:	PROJECT NUMBER:
Example 7000 x 12000 x 2400 Region B1 Hip Carport	6461
CLIENT:	DRAWN BY:
Example Customer	dm3 Solutions
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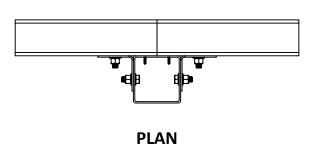
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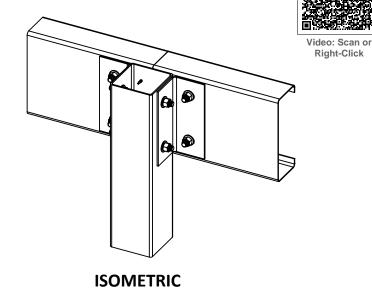
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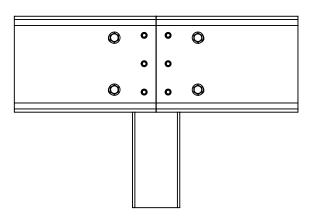
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2022-09-28	1	A4	NTS	18 of 27

#### **« Detail HC2.7: Fascia Beams to Post Connection Detail**









# **ELEVATION**

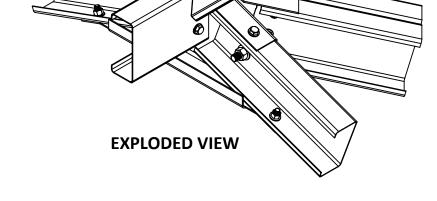


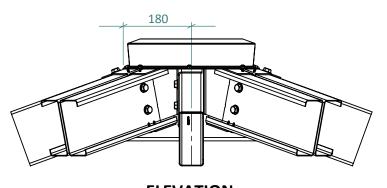
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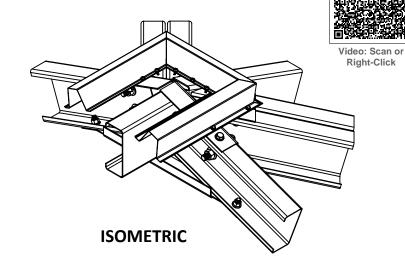


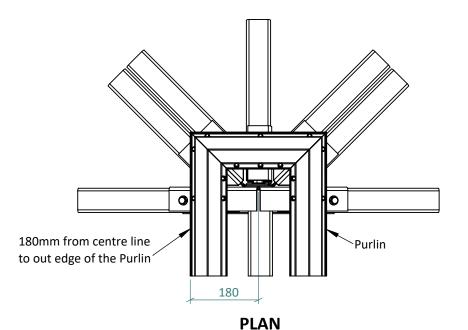
Purlin-

No.12-14x20 Tek Screw 2 per connection per intersection









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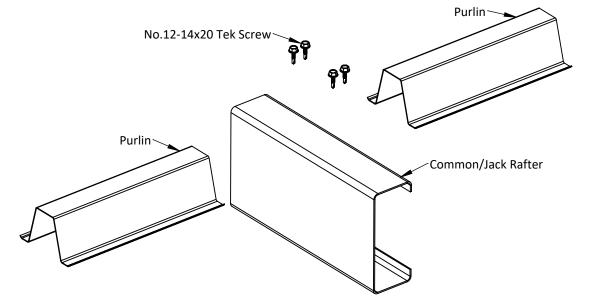
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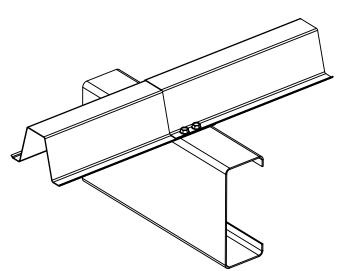
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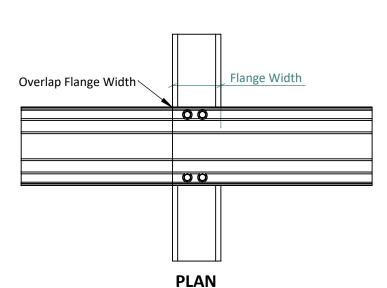


**EXPLODED VIEW** 



**ISOMETRIC** 

Example Street, Suburb NT 0820



A4

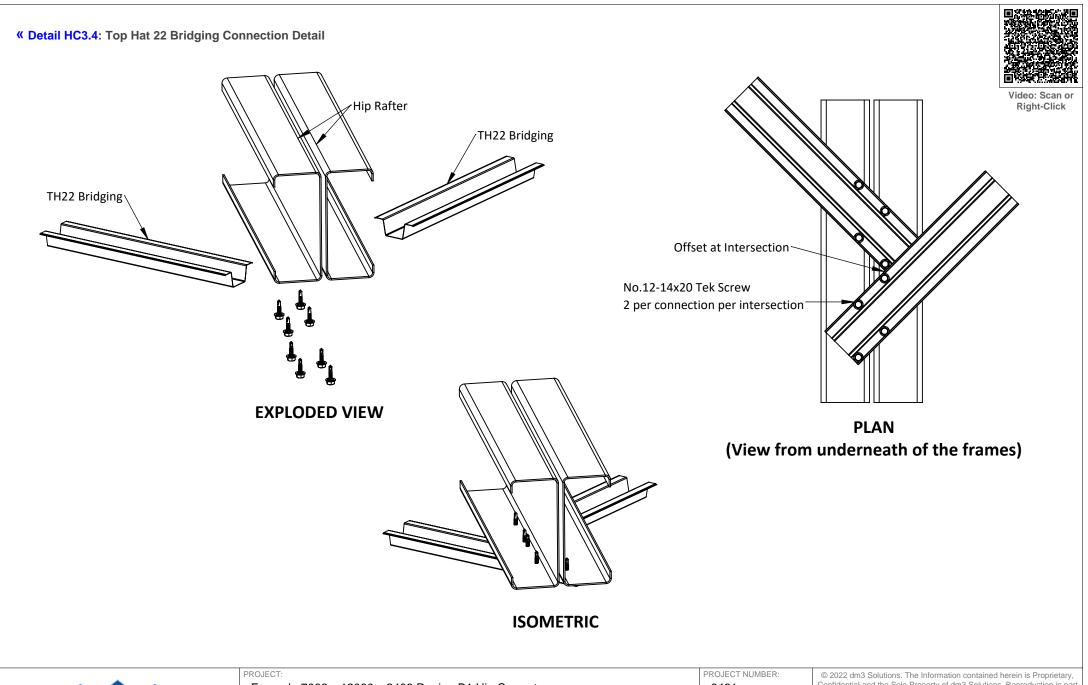
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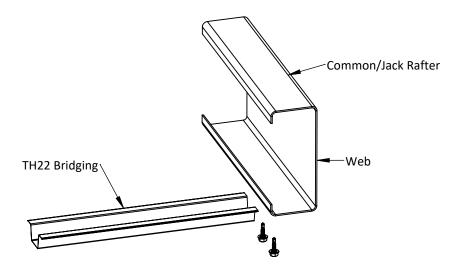




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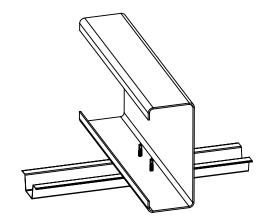
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No.12-14x20 Tek Screw
2 per connection per intersection
20mm Max. from Web

20 Max.

**EXPLODED VIEW** 



PLAN (View from underneath of the frames)

# **ISOMETRIC**

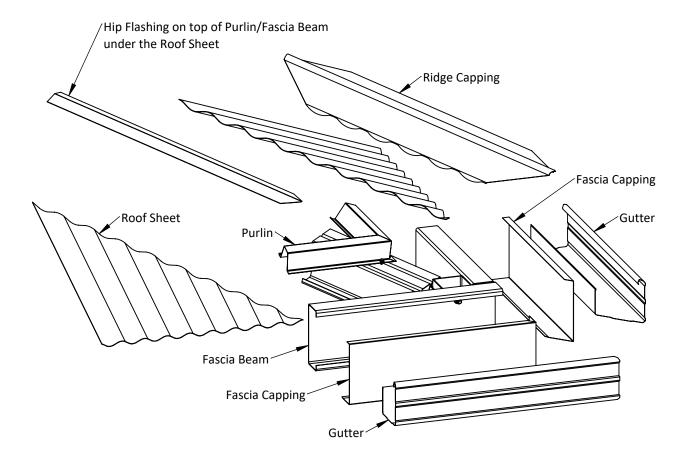


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# **« Detail HC4.1.1: Cladding Connection Detail**

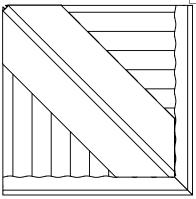


Video: Scan or Right-Click

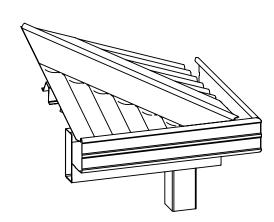




Example Street, Suburb NT 0820



**PLAN** 



# **ISOMETRIC**

2022-09-28

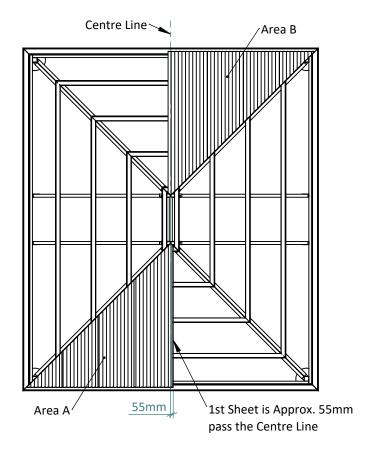
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Example 7000 x 12000 x 2400 Region B1 Hip Carport	6461
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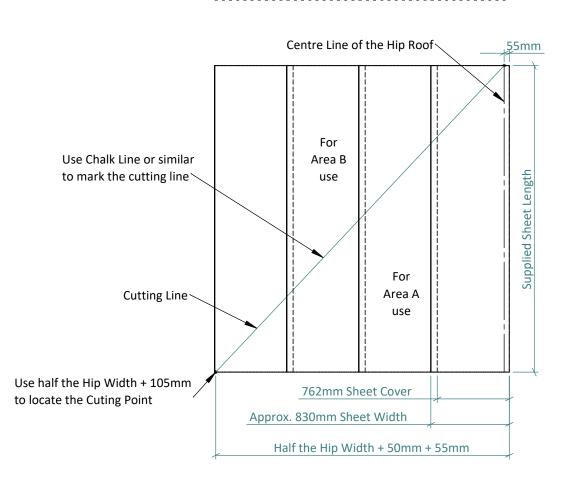
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A TYPICAL 6000 x 7000 HIP ROOF





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	Technical Data	CF . Reg	33.816 kN		Source: Lysaght® & Stramit® Span Tables			
Region	B1 · Regional Wind Speed 57 m/s		<b>56.268 kN</b> · L <sub>ex</sub> 2484 · L <sub>ey</sub> 2484 · L <sub>ez</sub> 2484	Def	Flection 1.8 mm · 8.8 mm Permitted			
Terrain Category	2.5	Common Rafter · CR · Calculations		H	Hip End Fascia Beam Calculations			
$ ilde{Q_{u}}$ Inward Load	No 47.11 m/s 1.332 kPa 0.826 kPa · max(0.495, 0.826, 0.12) 0.616 kPa · max(0.31, 0.616)	Min Yield Stress · Grade · I <sub>x</sub> Gov Span · Load Width <b>Capacity</b> · Inward Req		Min Yield Stress · G Go Concentrate	Member       C20019 $\text{frade} \cdot \text{I}_{\text{x}}$ 450 MPa · G450 · 4.51 x $10^6$ mm <sup>4</sup> ov Span       6748 mm         ed Load       One Point Load at 3374 mm			
		1	1.489 kN/m		ard Req 0.488 kN/m			
Fascia to Apex Left Elevation Area Front Elevation Area Post RF Load Area	13.038 m <sup>2</sup> 31.663 m <sup>2</sup> 5.277 m <sup>2</sup> · RF = Raking Force 10.554 m <sup>2</sup> · Int = Internal	Ø <b>M</b> · Inward Req Inward ØM Outward Req Outward ØM	<b>12.434 kNm</b> · L <sub>ex</sub> 3395 · L <sub>ey</sub> 883 · L <sub>ez</sub> 1132	Member C Ø <b>M</b> ∙ Inw	O.364 kN/m  1.460 kN/m · 1 row of Bridging · Span @ 6900 mm Source: Lysaght® and Stramit® Span Tables ard Req 2.777 kNm 2.072 kNm ØM 8.977 kNm · L <sub>ex</sub> 6748 · L <sub>ey</sub> 3374 · L <sub>ez</sub> 3374			
Int Post Uplift Area		Internal Com	mon Rafter · ICR · Calculations		ard Req 13.019 kN			
Base Connection	Post Calculations Cast In	Details · Member  Min Yield Stress · Grade · I <sub>x</sub> Gov Span · Load Width  C15024 with 2 rows of Bridging  450 MPa · G450 · 2.54 x 10 <sup>6</sup> mm <sup>4</sup> 6680 mm · 2500 mm		Combine	<b>27.747 kN</b> · L <sub>ex</sub> 6748 · L <sub>ey</sub> 3374 · L <sub>ez</sub> 3374 <b>IF</b> & M  Ratio must be <= 1.0 · AS4600 S3.5 <b>O.7</b>			
Uplift · End Post · Inward	450 MPa · Min = Minimum 6.154 kN	Capacity · Inward Req			Flection 2.8 mm · 22.5 mm Permitted			
-	4.589 kN		1.540 kN/m		a Beam Calculations for FB1.1 & FB1.2			
RF · End Post · Req Int Post · Req ØM · End Post · Req	9.179 kN 4.212 kN · Req = Required 8.425 kN	Ø <b>M</b> · Inward Req Inward ØM Outward Req Outward ØM	<b>12.434 kNm</b> $\cdot$ L <sub>ex</sub> 3395 $\cdot$ L <sub>ey</sub> 883 $\cdot$ L <sub>ez</sub> 1132	Min Yield Stress · G G Concentrate Capacity · Inw.	Member irrade · I <sub>x</sub> 450 MPa · G450 · 3.53 x 10 <sup>6</sup> mm <sup>4</sup> ov Span ed Load lard Req lard Rey lard Re			
Int Post · ØM Req 20.220 kNm			after · JR · Calculations		3900 mm			
Details · Member Min Yield Stress · Grade · I <sub>x</sub> Gov Span · Load Width Capacity · Inward Req	21.000 kNm · 100x100x4 Source: OneSteel® DuraGal® Hollow Sections  Ifter · HR · Calculations  C15012 with 2 rows of Bridging 500 MPa · G500 · 1.29 x 106mm4 4607 mm · 1167 mm · Gov = Governing	Details · Member Min Yield Stress · Grade · I <sub>x</sub> Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity	C15012 with 2 rows of Bridging 500 MPa · G500 · 1.29 x 10 <sup>6</sup> mm <sup>4</sup> 3415 mm · 1167 mm 0.964 kN/m 0.719 kN/m 2.880 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 1.405 kNm	Outwood CF · Outwood Components Components Components Components Combine	Source: Lysaght® & Stramit® Span Tables  2.437 kNm  1.818 kNm  ØM  6.487 kNm · L <sub>ex</sub> 3784 · L <sub>ey</sub> 3374 · L <sub>ez</sub> 3374  13.019 kN  18.870 kN · L <sub>ex</sub> 3784 · L <sub>ey</sub> 3374 · L <sub>ez</sub> 3374  Ratio must be <= 1.0 · AS4600 S3.5  ed Ratio  0.970  flection  0.8 mm · 12.6 mm Permitted			
·	1.620 kN/m · min(Inward, Outward)	1	<b>5.310 kNm</b> · L <sub>ex</sub> 3415 · L <sub>ey</sub> 883 · L <sub>ez</sub> 1138	Fascia	a Beam Calculations for FB2.1 & FB2.2			
Ø <b>M</b> · Inward Req Inward ⊘M Outward Req	<b>5.310 kNm</b> $\cdot$ L <sub>ex</sub> 4607 $\cdot$ L <sub>ey</sub> 883 $\cdot$ L <sub>ez</sub> 1536	Deflection	<b>5.310 kNm</b> $\cdot$ L <sub>ex</sub> 3415 $\cdot$ L <sub>ey</sub> 1138 $\cdot$ L <sub>ez</sub> 1138 <b>0.8 mm</b> $\cdot$ 11.4 Permitted	<b>Details</b> · I Min Yield Stress · G	Member C20015 with 1 row of Bridging rade $\cdot$ $I_x$ 450 MPa $\cdot$ G450 $\cdot$ 3.53 $\times$ 10 <sup>6</sup> mm <sup>4</sup> ov Span 3820 mm			
	<b>5.310 kNm</b> · L <sub>ex</sub> 4607 · L <sub>ey</sub> 1536 · L <sub>ez</sub> 1536		ning Purlin Calculations		ed Load 1 Point Load at 1910 mm			
CF · Inward Req Member L Compression CF & ØM Combined Ratio	<b>12.345 kN</b> · CF = Compression Force <b>25.56 kN</b> · L <sub>ex</sub> $4607 \cdot L_{ey} 883 \cdot L_{ez} 1536$ Ratio must be <= $1.0 \cdot AS4600 S3.5$	Gov Span · Load Width <b>Capacity</b> · Inward Req	Min Yield Stress · Grade · I Gov Span · Load Widt Capacity · Inward Re	550 MPa · G550 · 0.16 x 10 <sup>6</sup> mm <sup>4</sup> 2639 mm · 883 mm	Outw Member C	ard Req ard Req ard Req 2.693 kN/m 4.370 kN/m · 1 row of Bridging · Span @ 3900 mm Source: Lysaght® & Stramit® Span Tables ard Req 6.587 kNm		
Ridge Line Strut · RLS · Calculations		<b>→</b>	0.816 kN/m		ard Req 4.913 kNm			
Details · Member C15024  Min Yield Stress · Grade · I <sub>x</sub> Gov Span Gov Span 2484 mm		Outward Capacity	<b>1.380 kN/m</b> · Single Span @ 3000 mm · Minimum Support Thickness of 1.2 mm BMT 2 x No. 12 Tek Screws	CF · Outw	ØM <b>9.761 kNm</b> · L <sub>ex</sub> 3820 · L <sub>ey</sub> 1910 · L <sub>ez</sub> 1910 ard Req <b>13.019 kN</b>			
^^	PROJECT: Example 7000 x 120 CLIENT:	000 x 2400 Region B1 Hip	Carport	PROJECT NUMBER: 6461  PRAWN BY:	© 2022 dm3 Solutions. The Information contained herein is Proprietary, Confidential and the Sole Property of dm3 Solutions. Reproduction in part or in whole is Prohibited without written approval. Unless specified otherwise, dimensions are in millimetres & drawings are not to scale.			
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Example Street, Suburb NT 0820

Member L Compression **48.621 kN** · L<sub>ex</sub> 3820 · L<sub>ev</sub> 1910 · L<sub>ez</sub> 1910 **CF** &  $\emptyset$ **M** Ratio must be  $\leq 1.0 \cdot AS4600 S3.5$ Combined Ratio 0.771 **Deflection 2.7 mm** · 12.7 mm Permitted Fascia Beam Calculations for FB3.1 & FB3.2 **Details** · Member **C20015** with 1 row of Bridging Min Yield Stress · Grade · I<sub>x</sub> 450 MPa · G450 · 3.53 x 10<sup>6</sup>mm<sup>4</sup> Gov Span 3784 mm Concentrated Load 1 Point Load at 410 mm Capacity · Inward Reg 1.362 kN/m Outward Req 1.016 kN/m Member Capacity 4.370 kN/m ⋅ 1 row of Bridging ⋅ Span @ 3900 mm Source: Lysaght® & Stramit® Span Tables ØM · Inward Req 2.437 kNm Outward Req 1.818 kNm ØM **6.487 kNm** · L<sub>ex</sub> 3784 · L<sub>ev</sub> 3374 · L<sub>ez</sub> 3374 CF · Outward Req 13.019 kN Member L Compression **18.870 kN** · L<sub>ex</sub> 3784 · L<sub>ev</sub> 3374 · L<sub>ez</sub> 3374 **CF** &  $\emptyset$ **M** Ratio must be  $\leq 1.0 \cdot AS4600 S3.5$ Combined Ratio 0.970 **Deflection 0.8 mm** · 12.6 mm Permitted **Cladding Details** Product BlueScope® Colorbond® Min Yield Stress · Grade 550 MPa · G550 Profile CustomOrb® BMT **0.42 mm** 

#### **End Posts Bored Piers**

ØM · M<sub>...</sub> 10.109 kNm Uplift · V<sub>...</sub> 4.589 kN Shear · Hu 4.212 kN

Dimensions 350 mm diameter x 1100 mm deep

#### **Internal Posts Bored Piers**

 $\emptyset$ M · M<sub>u</sub> 20.220 kNm Uplift · V. 9.179 kN Shear · H<sub>u</sub> 8.425 kN

Dimensions 450 mm diameter x 1300 mm deep

#### **Connection Calculations**

Connection Type Hip Rafter to Post Components 2 x M12 4.6/S Bolts Capacity Req 4.441 kN

Capacity **30.2 kN** · 2 x M12 4.6/S Shear Capacity

Connection Type Common Rafter to Fascia Components 2 x M12 4.6/S Bolts

Capacity Req 8.313 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type Internal Common Rafter to Fascia Components 2 x M12 4.6/S Bolts

Capacity Req 6.897 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type L/R Fascia Beam to Post

Components 2 x M12 4.6/S Bolts

Capacity Req 1.647 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type FB1.1 & FB1.2 to Post Components 2 x M12 4.6/S Bolts

Capacity Reg 2.577 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type FB2.1 & FB2.2 to Post Components 2 x M12 4.6/S Bolts

Capacity Reg 6.897 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type FB3.1 & FB3.2 to Post Components 2 x M12 4.6/S Bolts

Capacity Req 2.577 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

1 x M12 4.6/S Bolt Web Bearing Capacity

**24.883 kN** · 1.5 mm Gauge C-Section **31.519 kN** · 1.9 mm Gauge C-Section 39.813 kN · 2.4 mm Gauge C-Section

**Plate Bearing Capacity** 

**49.766 kN** · 3.0 mm Gauge Plate

1 x M16 8.8/S Bolt Web Bearing Capacity

53.084 kN · 2.4 mm Gauge C-Section 66.355 kN · 3.0 mm Gauge C-Section **Plate Bearing Capacity 66.355 kN** · 3.0 mm Gauge Plate

**Plates Tear Out Capacities** 

Minimum · Top Bolt 51.840 kN Minimum · Bottom Bolt 77.760 kN



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