

General Notes		Steel Grades		Design Notes & Structural Members				
G01	The Design and Details shown in these Drawings are applicable to this Project only.	300 PLUS	Universal Beams & Columns, Parallel Flange Channels, Large Angles to AS/NZS3679.1	Dimensions	7000 x 12000 x 2400			
G02	These Drawings shall be read in conjunction with all Architectural Drawings, other Consultants' Drawings, Specifications and such other Written Instructions as may be issued during the course of the Project. Any discrepancy shall be referred to the Engineer before proceeding with the work.	250	Flats, Small Angles, Taper Flange Beams & Columns to AS/NZS3679.1	Building Class	10a			
		300	Welded Sections to AS/NZS3679.2	Design Wind Speed	47.11 m/s			
G03	All Materials and Workmanship shall be in accordance with the relevant and current SAA Codes and Authorities Except where varied by the Project Specifications.	250	Hot Rolled Plates, Floor Plates & Slabs to AS/NZS3678	Columns C	See Post & Footing Layout			
G04	The Structure must be maintained in a stable condition and no part must be overloaded during construction. Temporary bracing must be designed and provided by the contractor(s) to keep the building works and excavations stable at all times.	250	Hollow Sections to AS1163. Circular Sections less than 165mm Outside Diameter. Sections other than the above	Rafters R	See Rafter Layout			
		G450-G550	Cold Form AS4600. Unless noted otherwise, all purlins, plates & brackets are G450.	Purlins P	TH6110 G550			
G05	The Issuer of these Designs reserves the right to alter Specifications and Designs as it may see fit without prior notification or penalty.	G500	Slab mesh and deformed reinforcement bars.	Fascia Beam FB	See Fascia Beam Layout			
G06	The Structure has not been designed for snow loads.	G2070	AS2841. Galvanized Steel Wire Strand. Unless noted otherwise, all Cable Bracing is G2070.	Footings	Cast In			
G07	Connections may require on-site drilling by the contractor(s).			All Brackets	3.0 mm G450			
				Technical Data	Page 25			
Foundation Notes								
F01	Foundations have been designed assuming an allowable bearing capacity of 100 kPa.	AS/NZS 1163	Cold-formed Structural Steel Hollow Sections					
F02	The Owner is advised to obtain a Geotechnical Report and/or Site Foundation Report to confirm F01 above.	AS/NZS 1170.0	Structural Design Actions - Part 0: General Principles					
F03	Footings shall be located centrally under walls and columns unless noted otherwise.	AS/NZS 1170.1	Structural Design Actions - Part 1: Permanent, Imposed & Other Actions					
F04	All Footings to be founded a minimum of 400 mm into Natural Ground. Do not Found Footings in uncontrolled fill.	AS/NZS 1170.2	Structural Design Actions - Part 2: Structural Design, Wind Actions					
F05	Engineer to be contacted if Foundation Conditions vary from F01 above.	AS/NZS 1170.4	Structural Design Actions - Part 4: Earthquake Actions in Australia					
		AS 1252	Structural Assemblies					
		AS/NZS 1554	Structural Steel Welding					
		AS 4100	Steel Structures					
		AS/NZS 4600	Cold-formed Steel Structures					
		AS/NZS 4671	Steel Reinforcing Materials					
		AS/NZS 4680	Hot-dip Galvanized (Zinc) Coatings on Fabricated Ferrous Articles					
		Region & Site Details						
		This is a freestanding roof only structure. It is assumed that cars, goods or materials stored under the roof block less than 50 percentage of the cross section exposed to the wind.						
		Suitable for Regions A1-5 & B1. TC2.5 without Shielding: Terrain with some trees or isolated obstructions, terrain in developing outer urban areas with scattered houses, or large acreage developments with more than two and less than 10 buildings per hectare. The site is not on the slope or crest of a hill, ridge or escarpment.						
		$V_{design} = V_R \times M_c \times M_d \times (M_{z,Cat} \times M_s \times M_t)$						
		$V_{design} = 57 \times 1 \times 0.95 \times (0.87 \times 1 \times 1) = 47.11 \text{ m/s}$						
		Foundations have been designed assuming an allowable bearing capacity of 100 kPa, undrained cohesion C _u = 50 kPa. Concrete Strength = 25 MPa.						

Left Elevation

7000 Frame Depth


20.0° Pitch

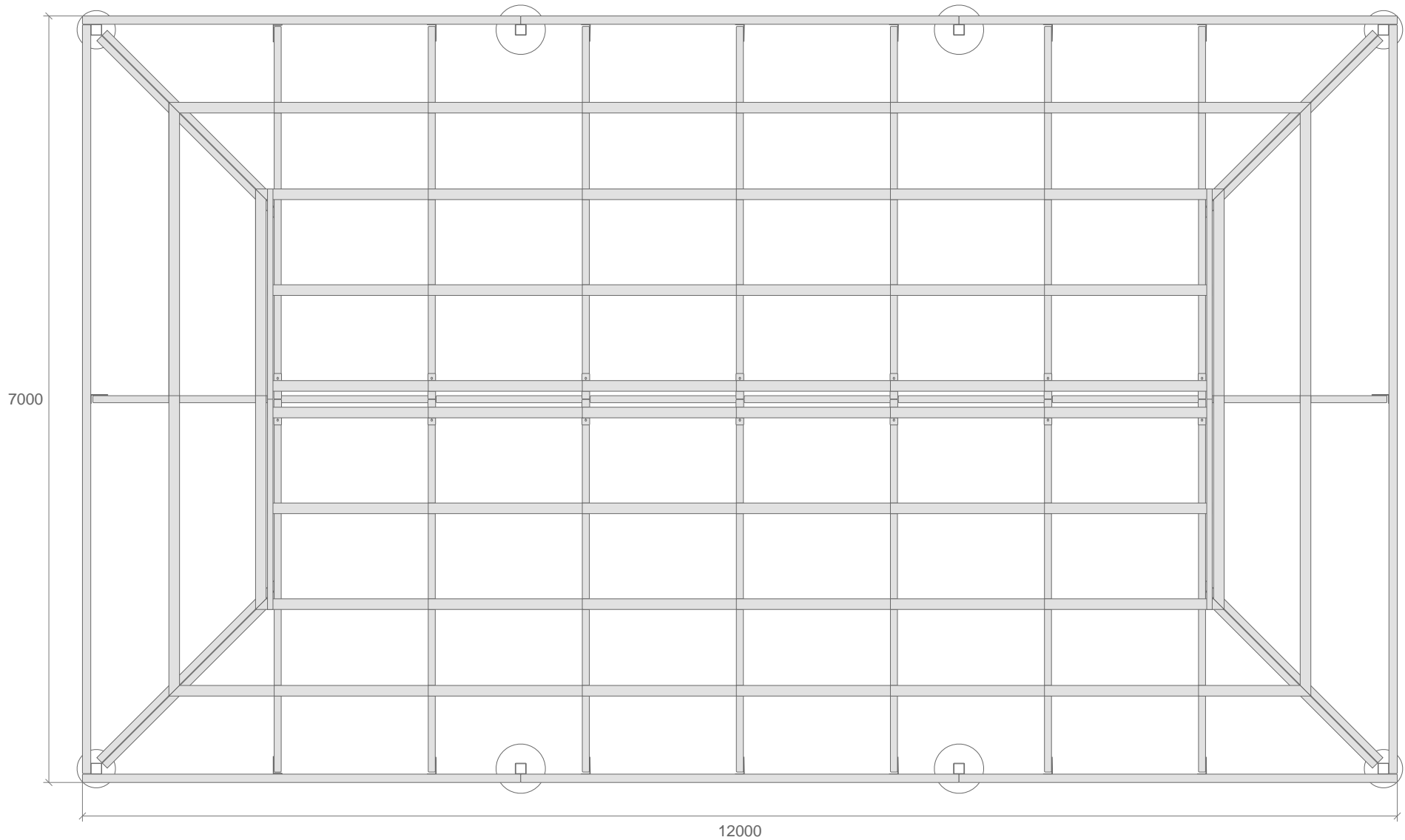
HC4.5.1


12000 Frame Width

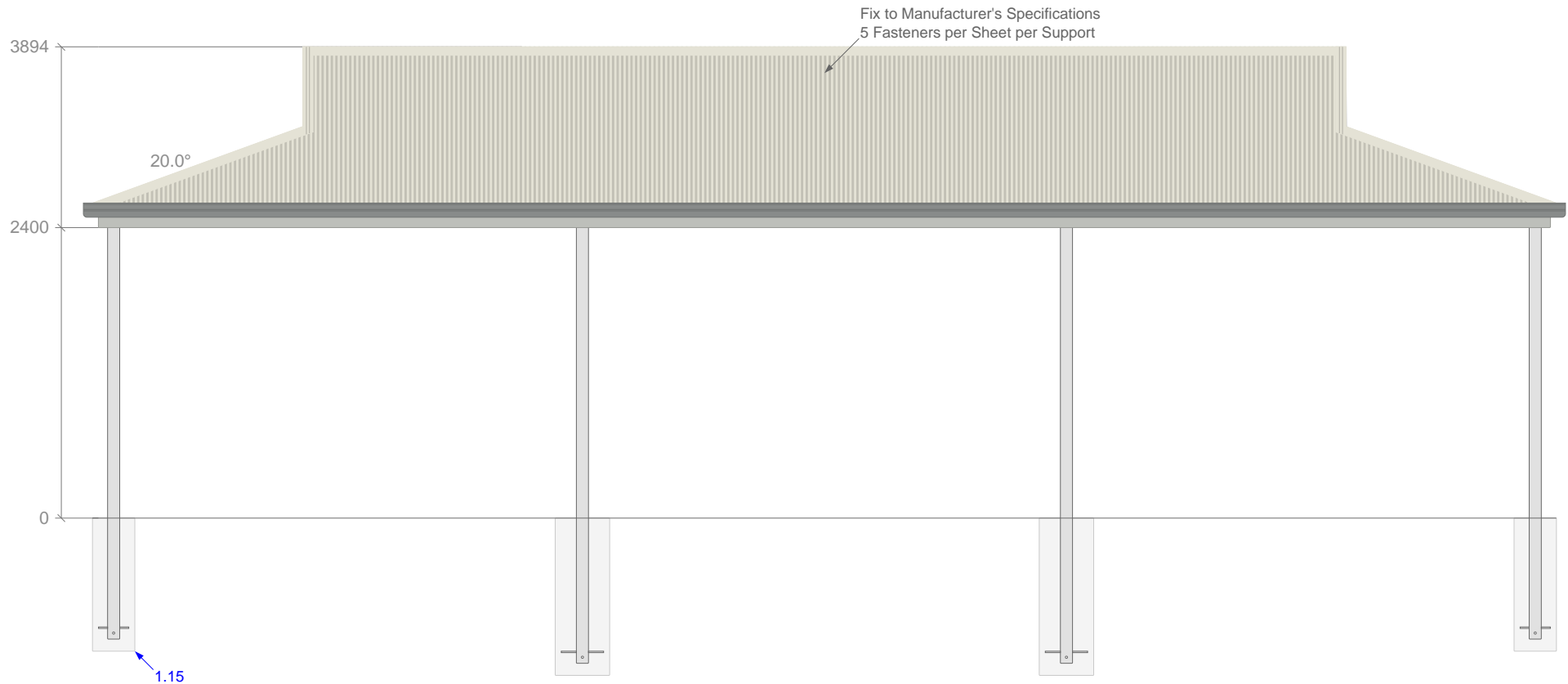
Front Elevation


Note: Roof structure is light foot trafficable for maintenance purposes.

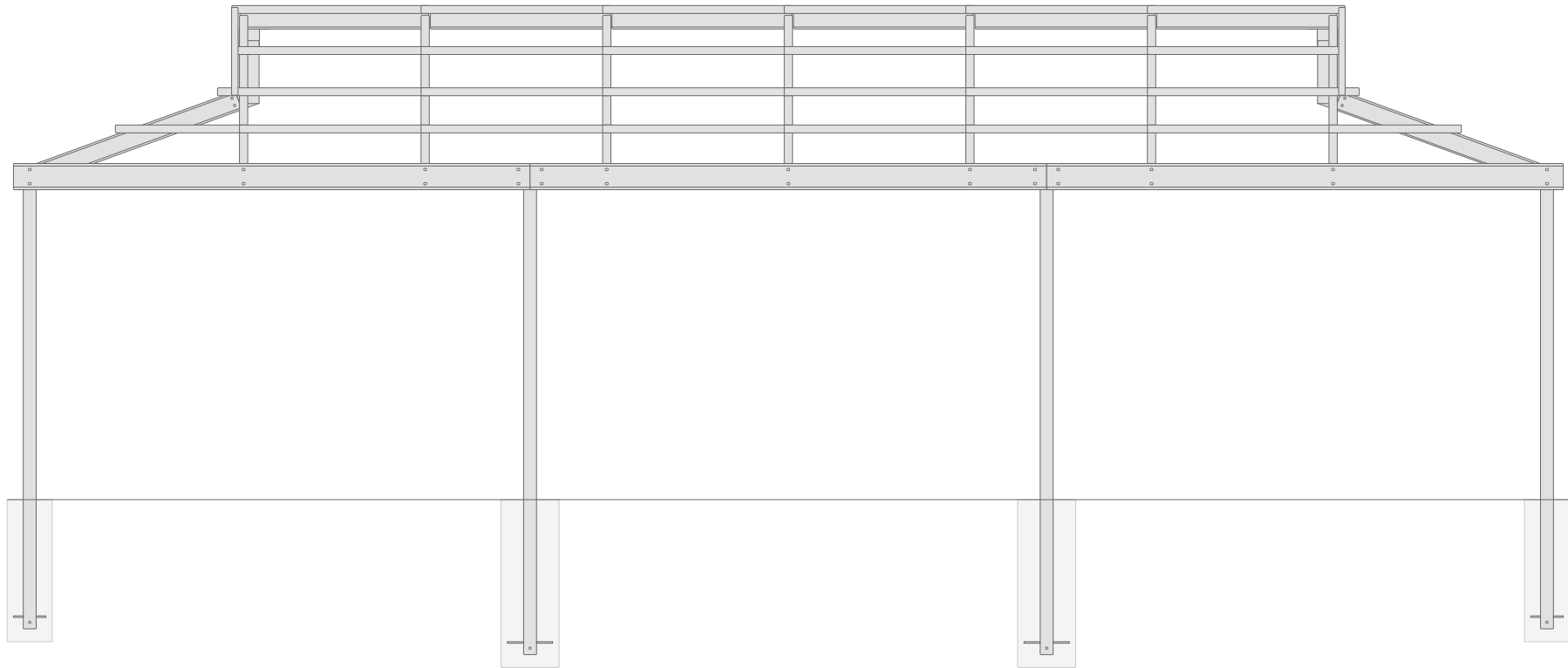
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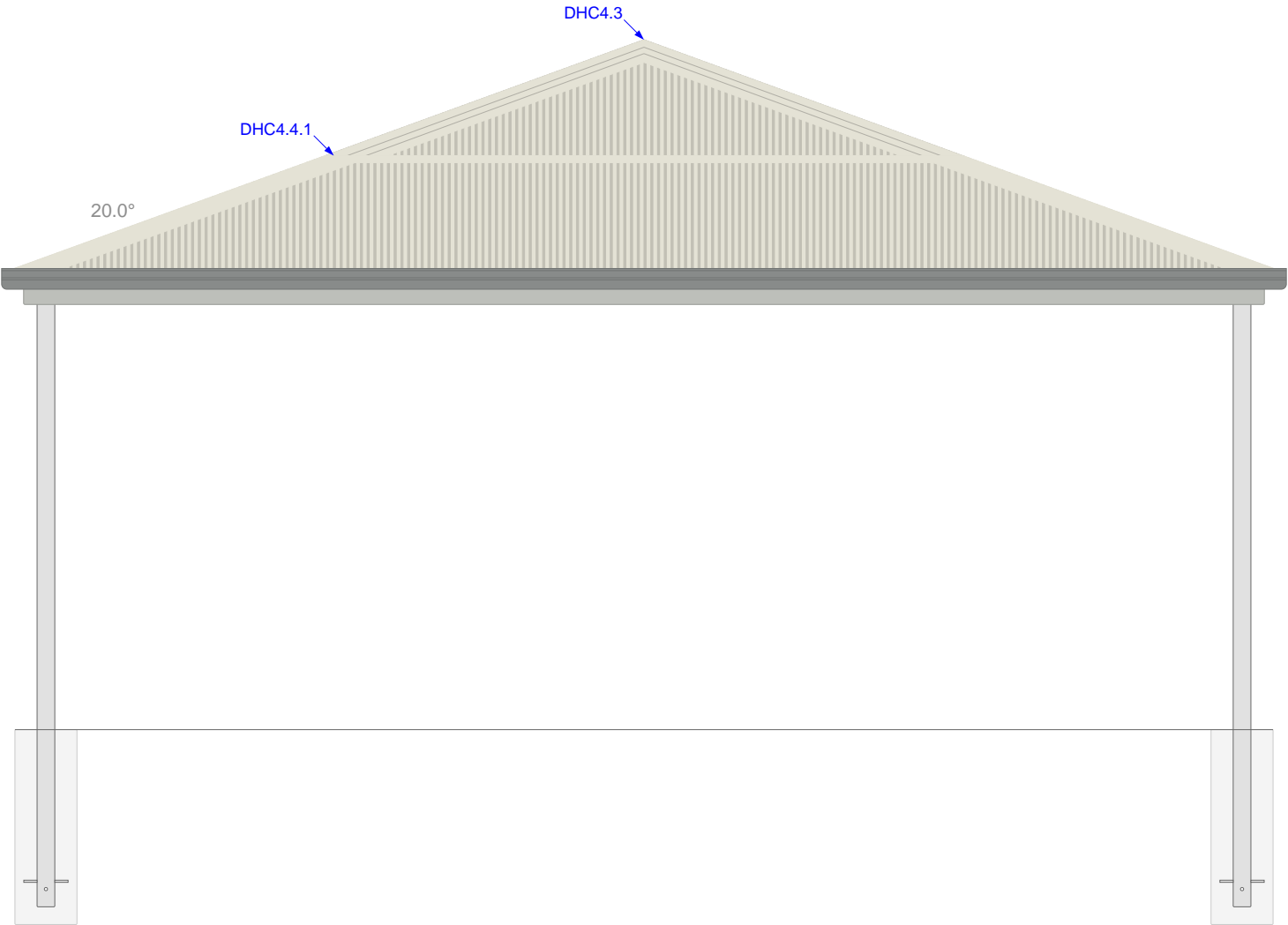
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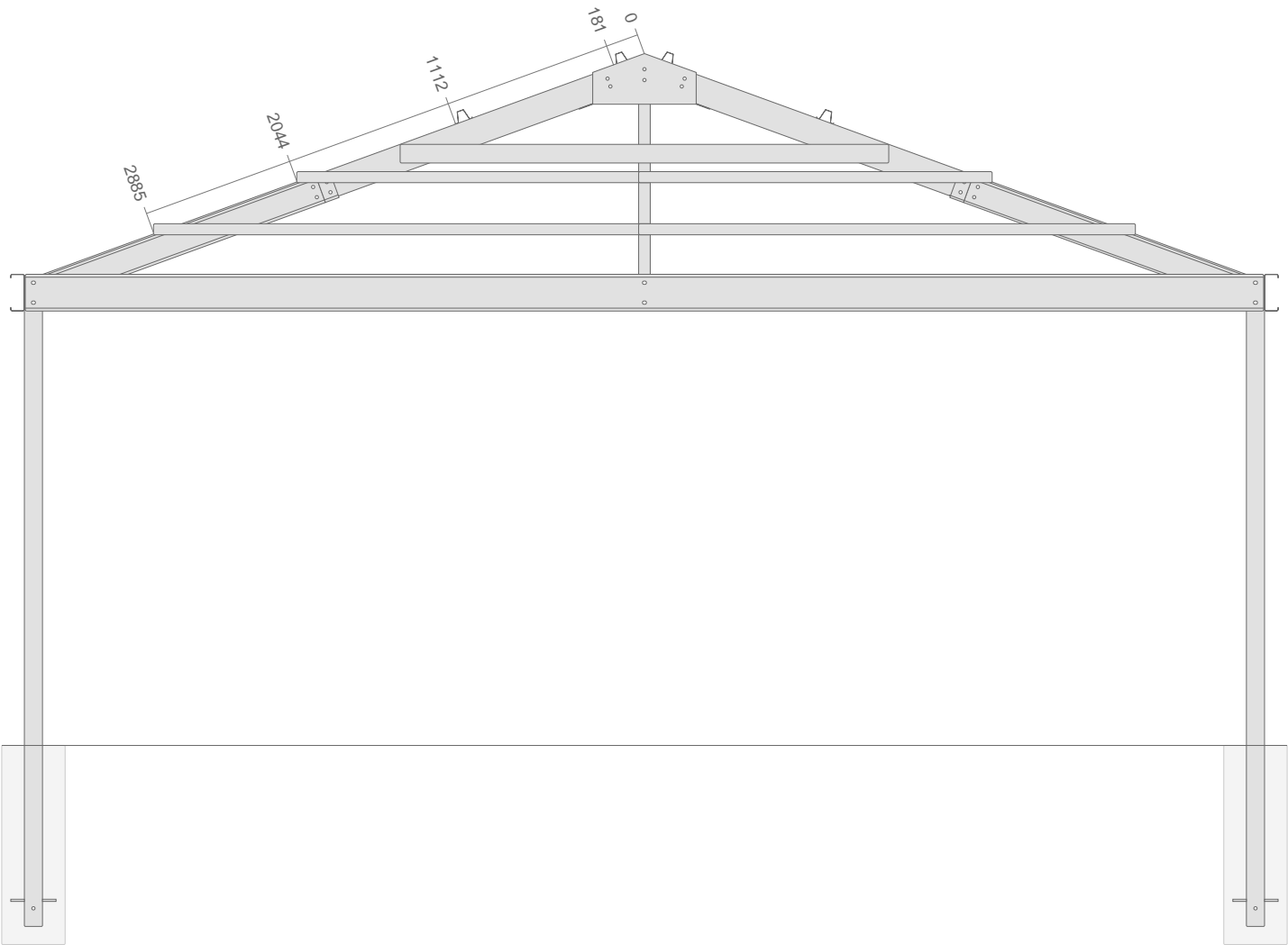
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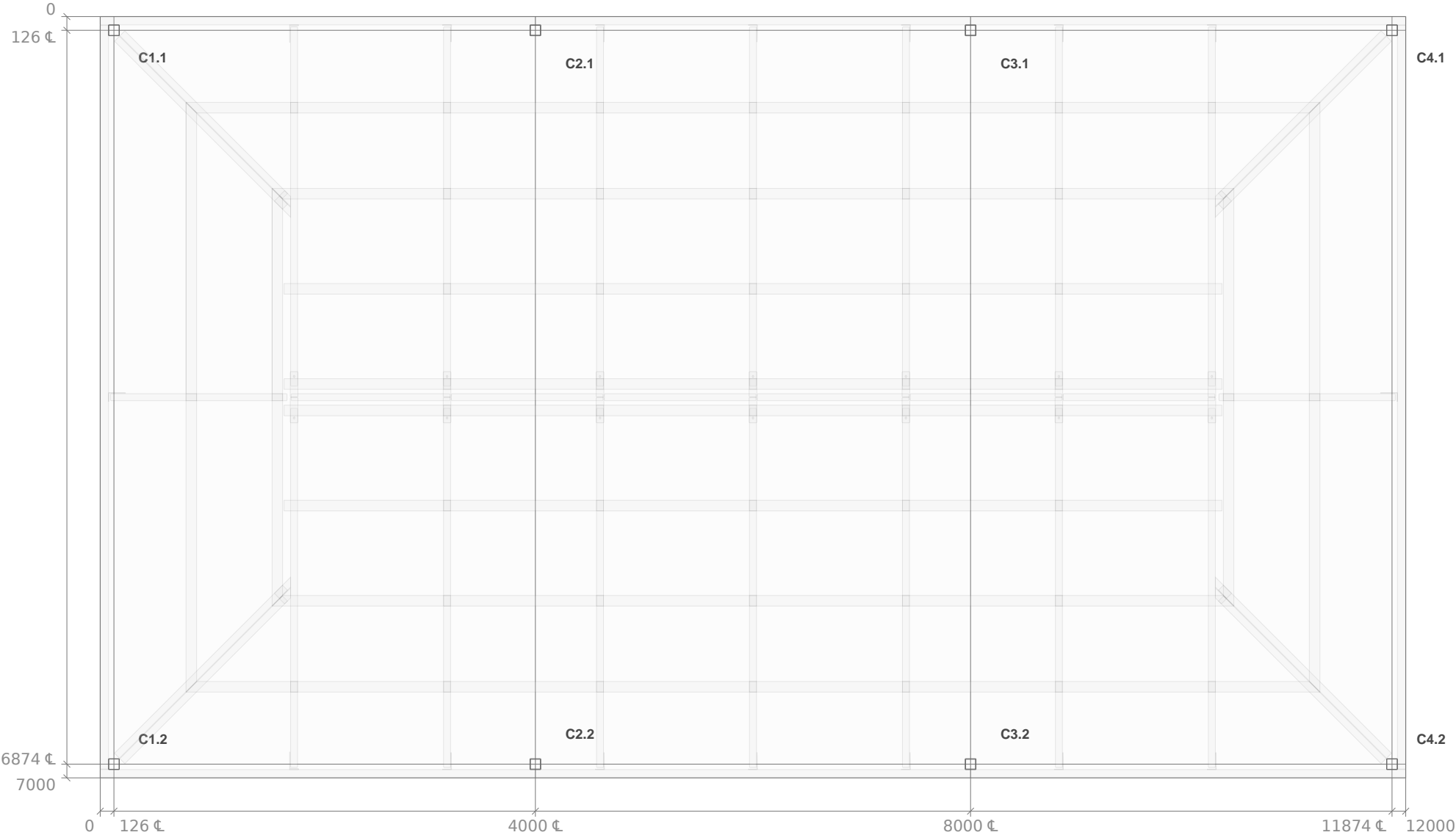



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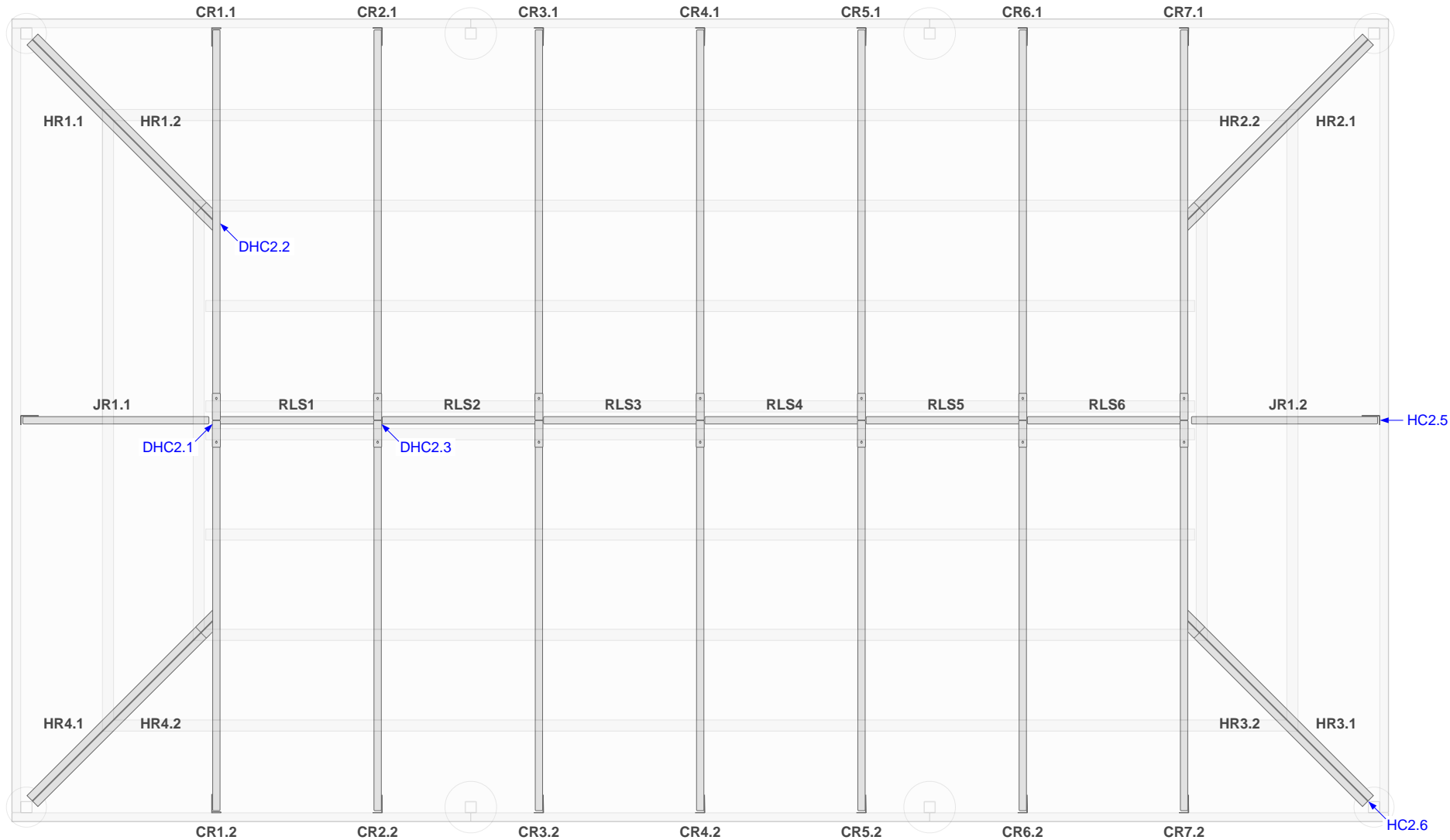


Dutch Gable Infill Members: TH6110 Infill Girt @ 2698 mm & C15012 Mullion @ 487 mm


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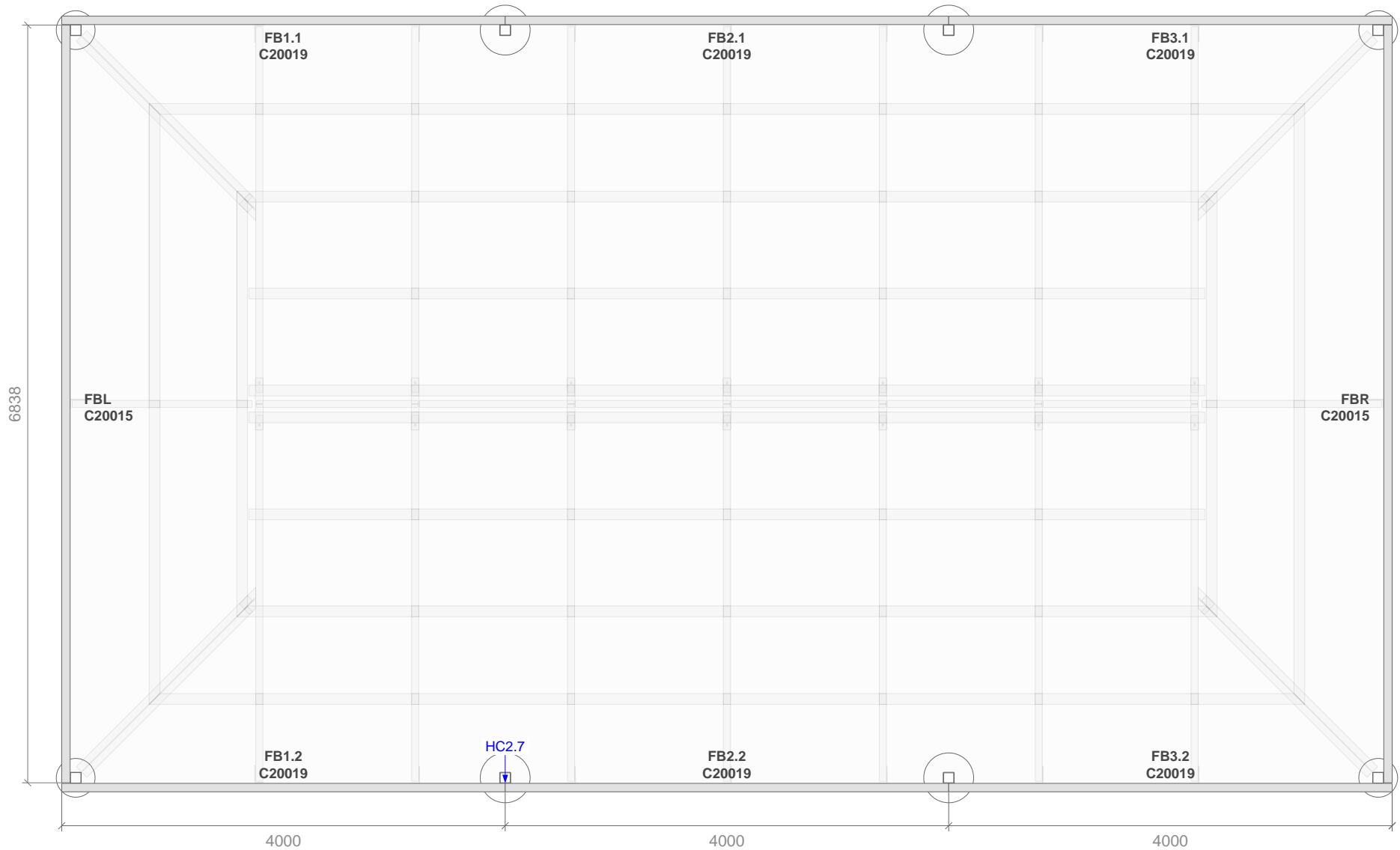



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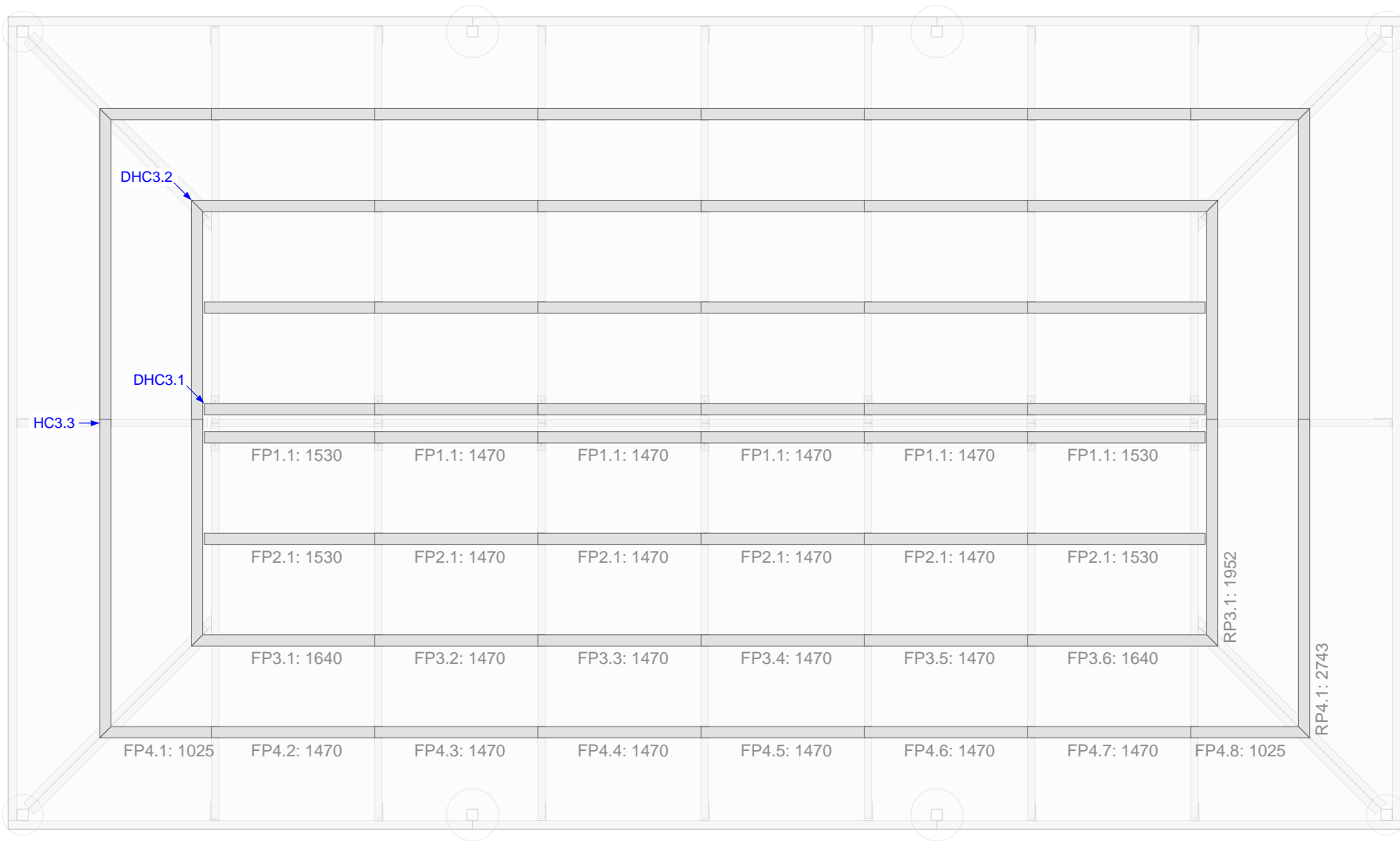


CR = Common Rafter · C15024 JR = Jack Rafter · C15012 Note: All Rafter to Fascia Connections
HR = Hip Rafter · C15012 RLS = Ridge Line Strut · C15012 Use Detail HC2.5 Above

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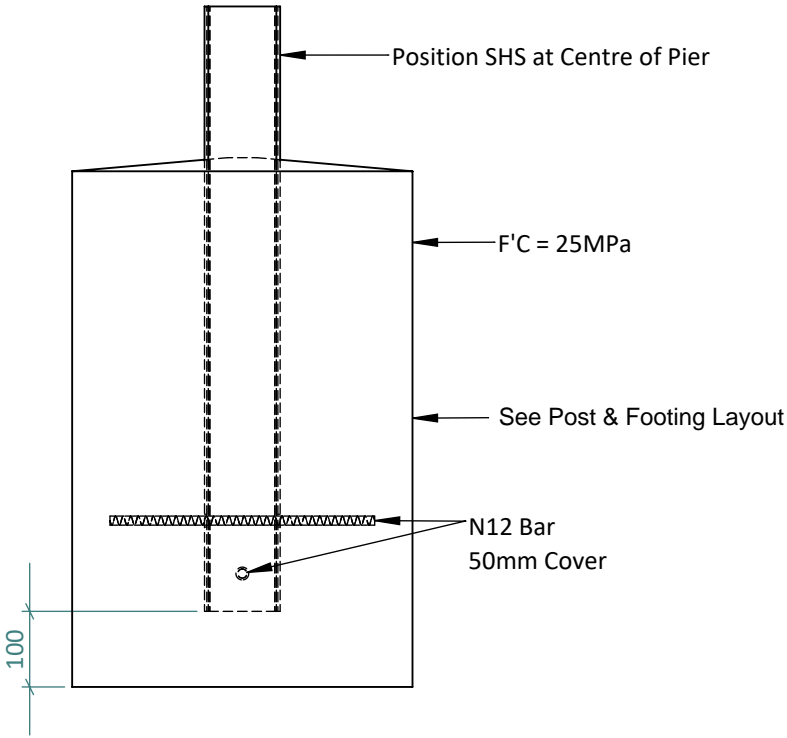


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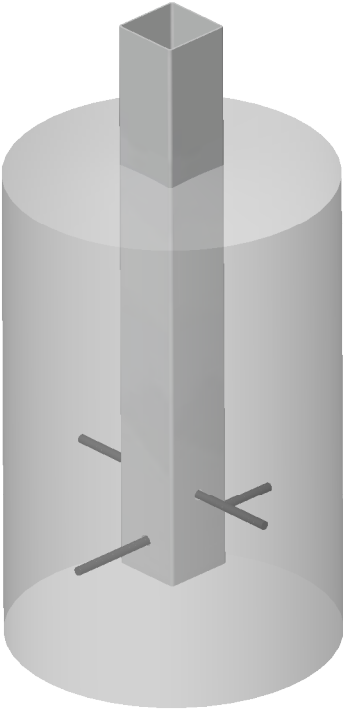




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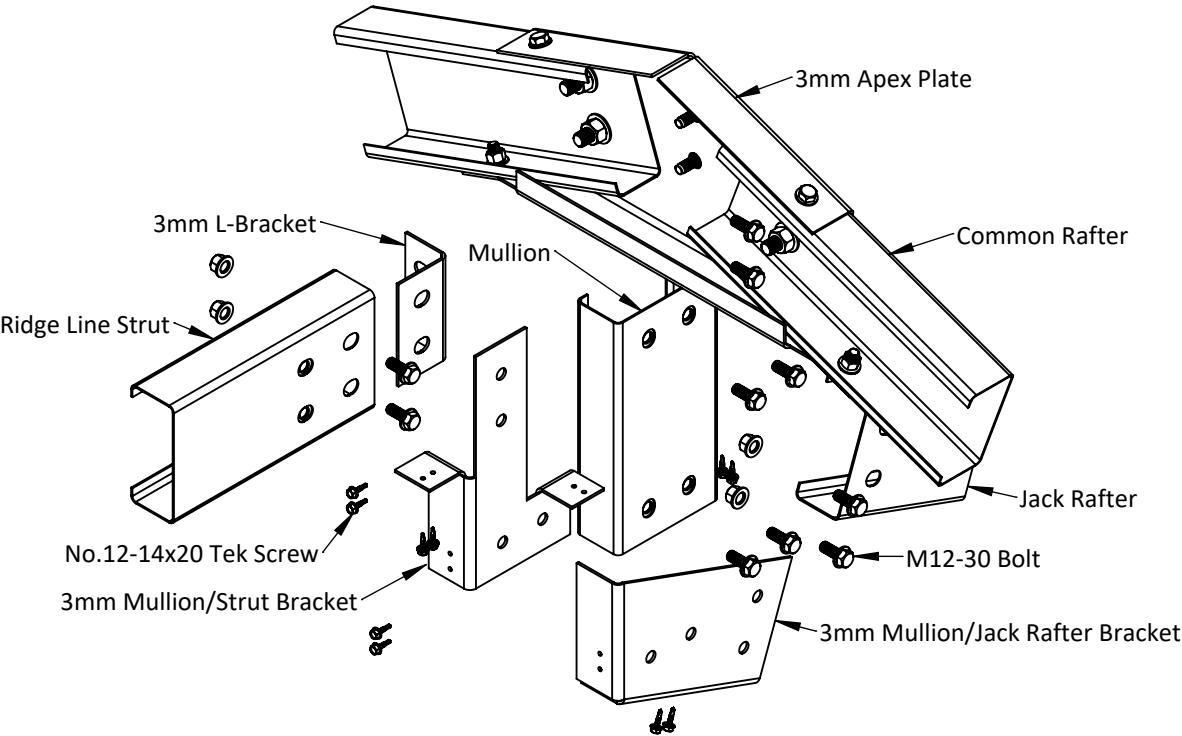
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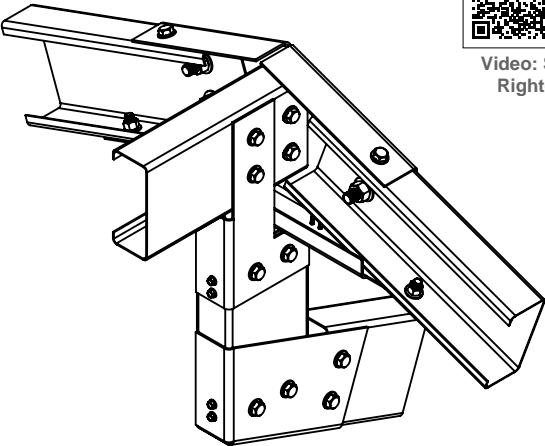
« Detail DHC2.1: Common Rafter Apex Connection Detail



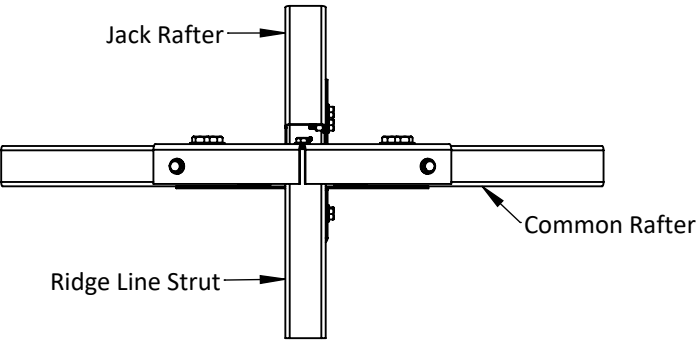
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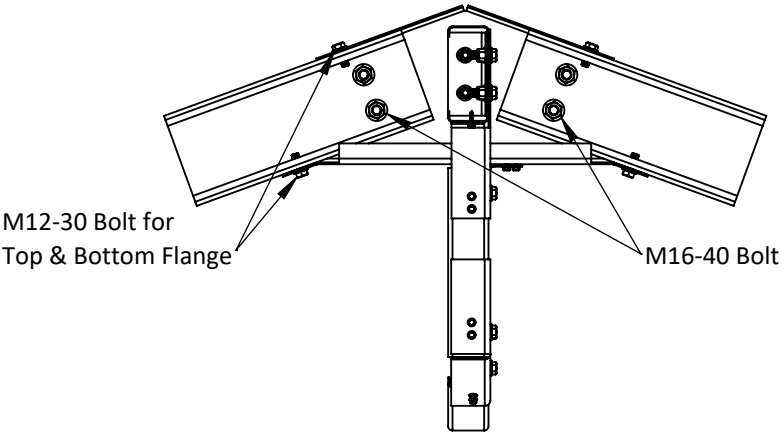
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PROJECT:
Example 7000 x 12000 x 2400 Region B1 Dutch Gable Carport

CLIENT:
Example Customer

ADDRESS:
Example Street, Suburb VIC 3333

PROJECT NUMBER:
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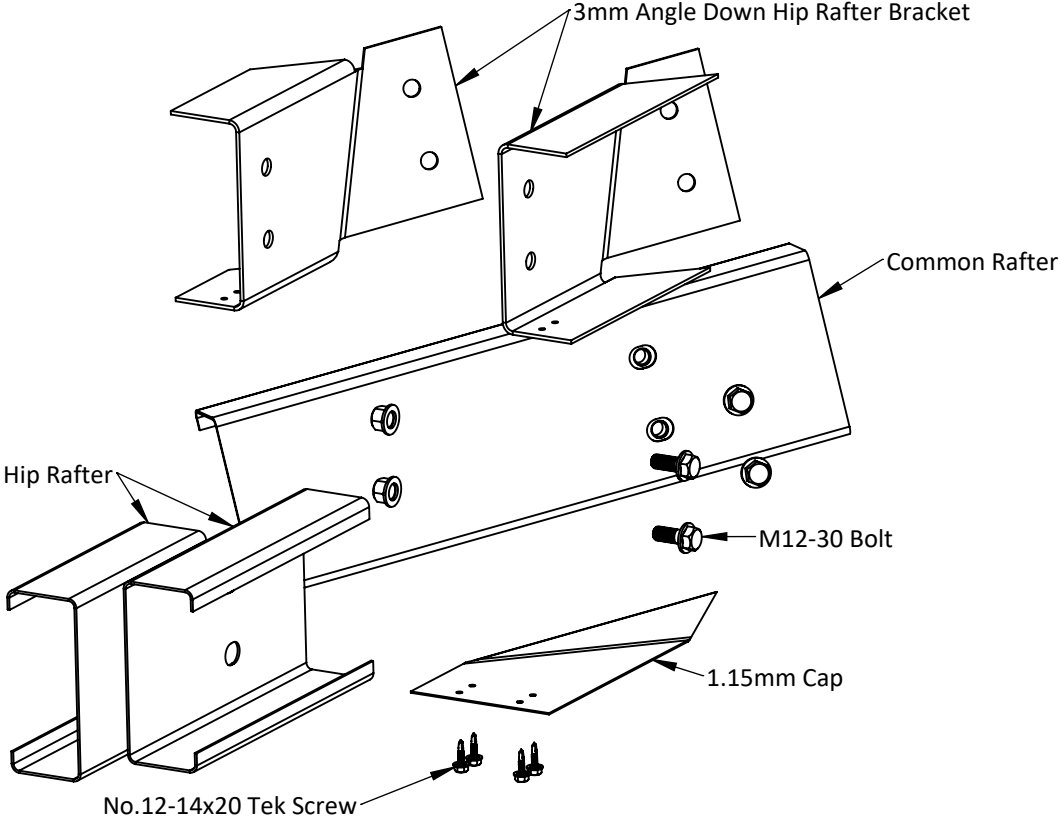
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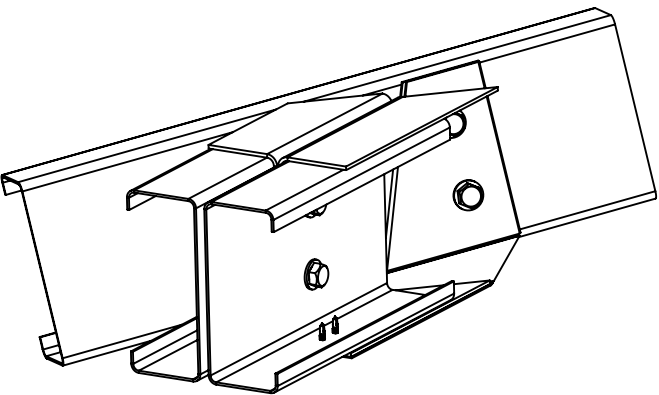
« Detail DHC2.2: Hip Rafter to Common Rafter Connection Detail



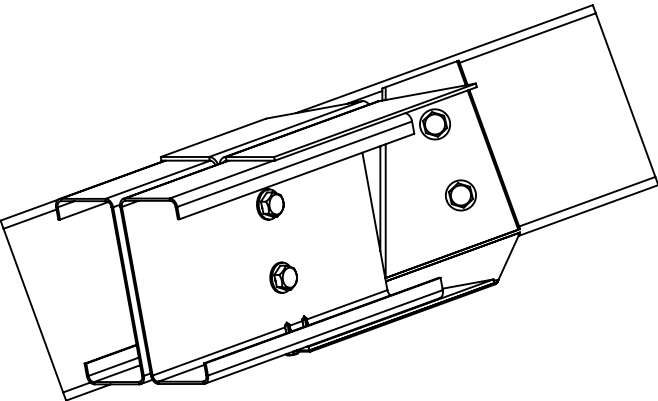
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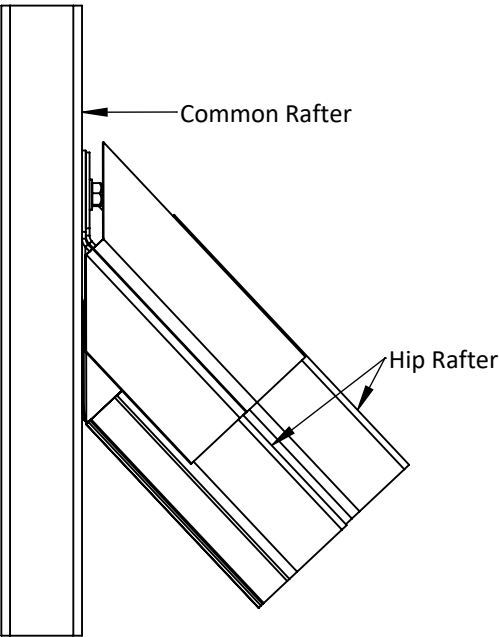
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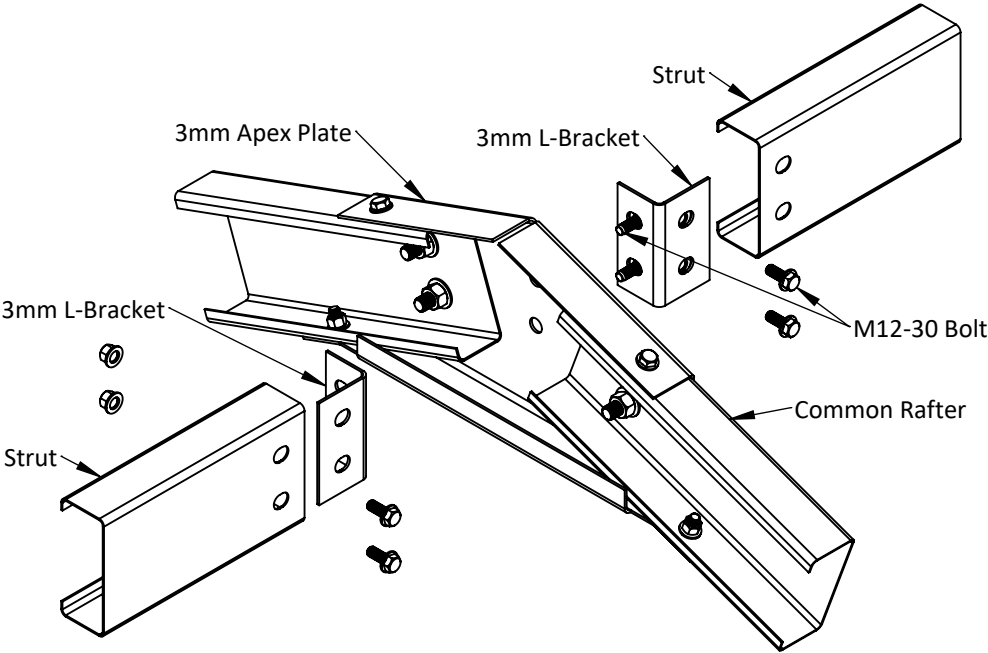


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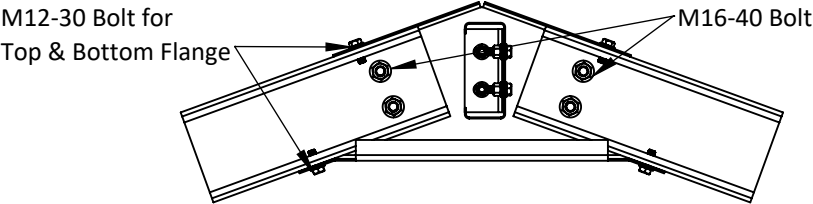
« Detail DHC2.3: Ridge Line Strut Apex Connection Detail



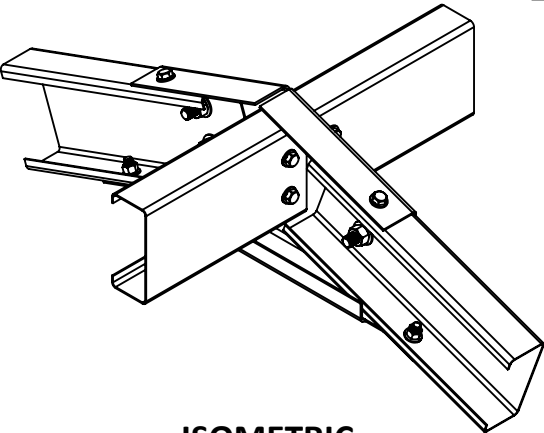
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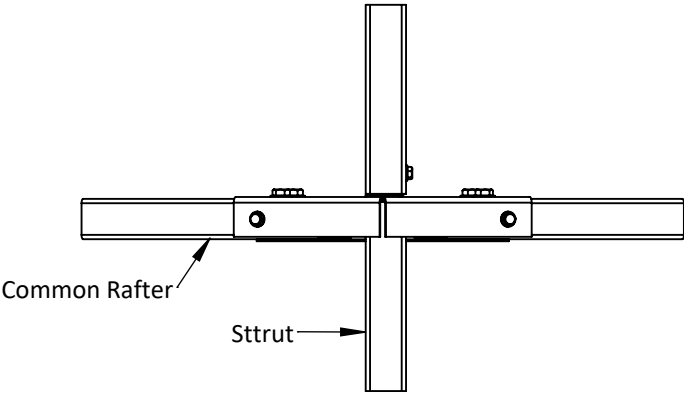
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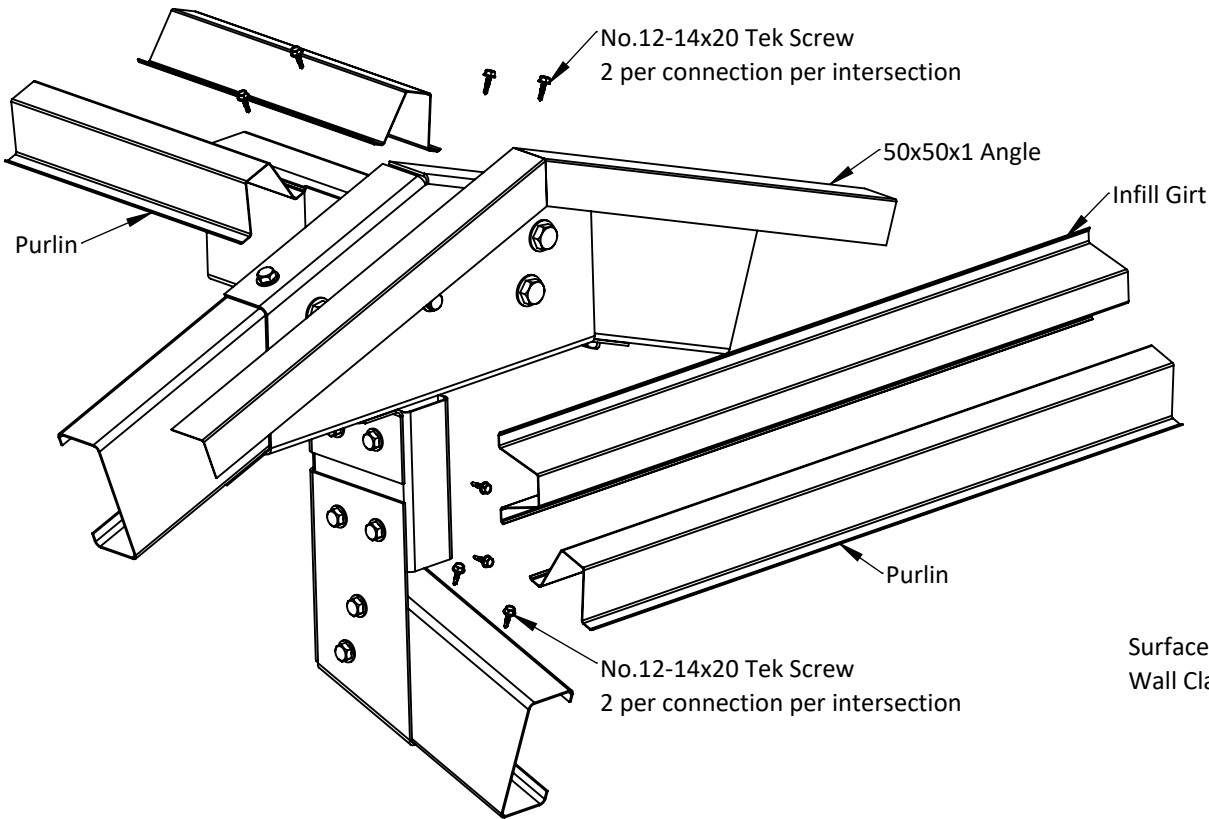
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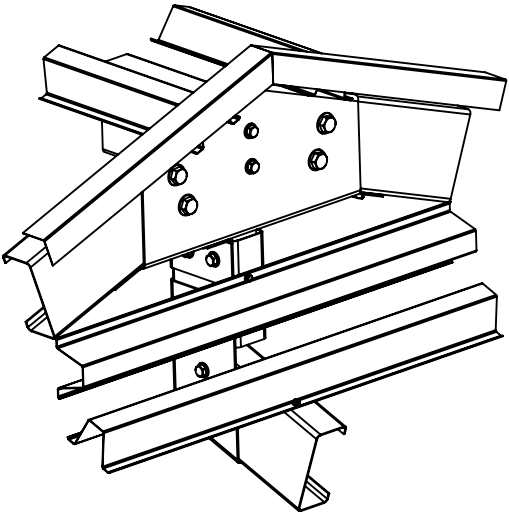
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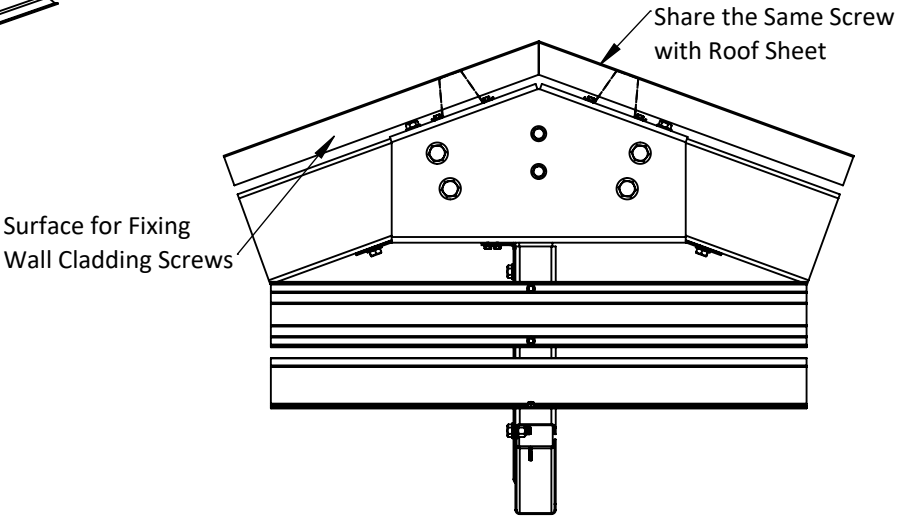
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PROJECT:
Example 7000 x 12000 x 2400 Region B1 Dutch Gable Carport

CLIENT:
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ADDRESS:
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PROJECT NUMBER:
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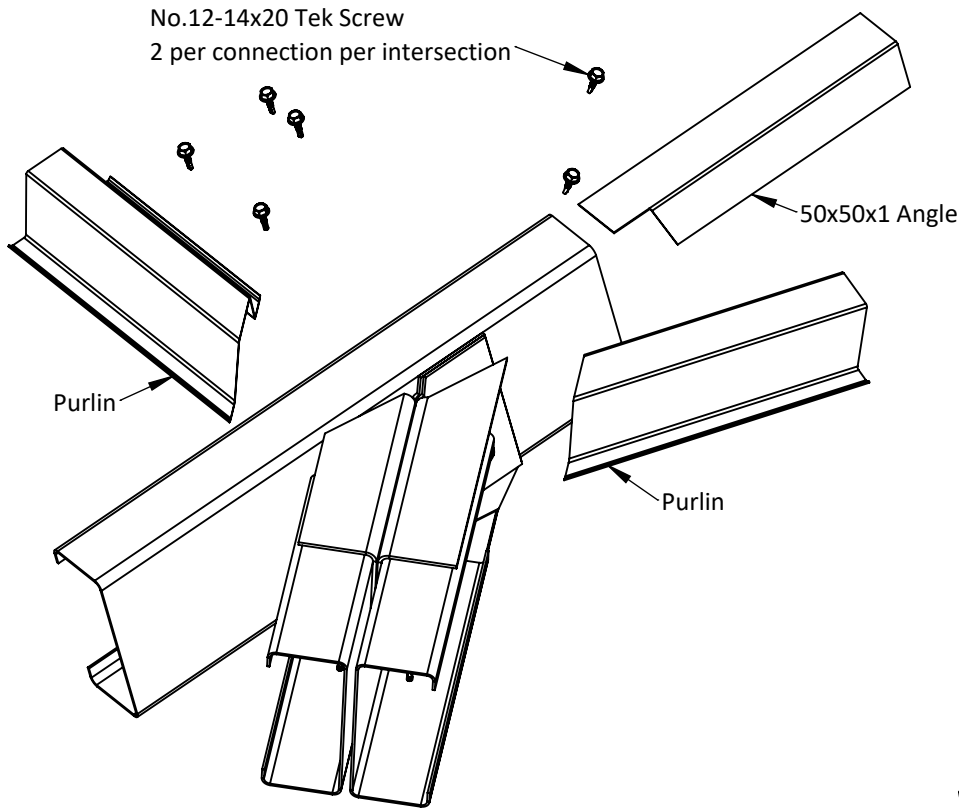
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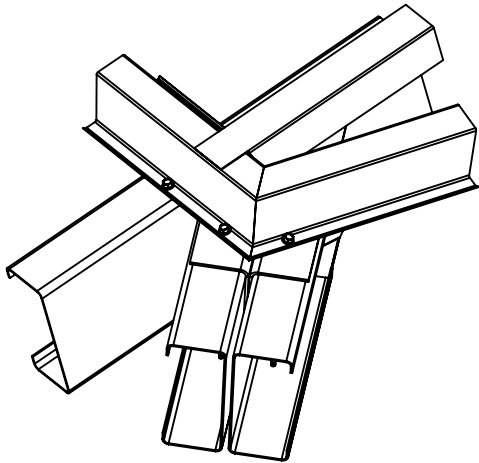
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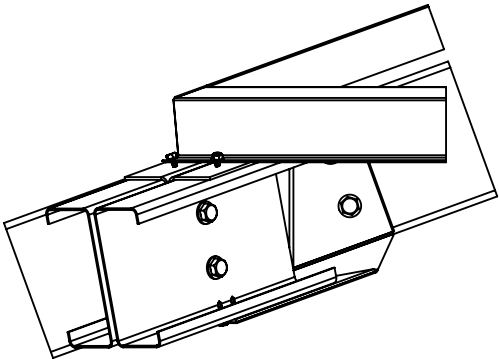
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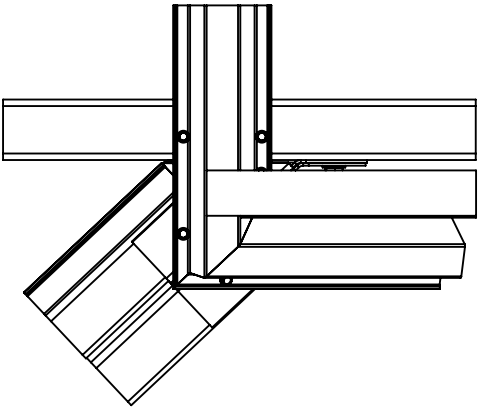
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PROJECT:
Example 7000 x 12000 x 2400 Region B1 Dutch Gable Carport

CLIENT:
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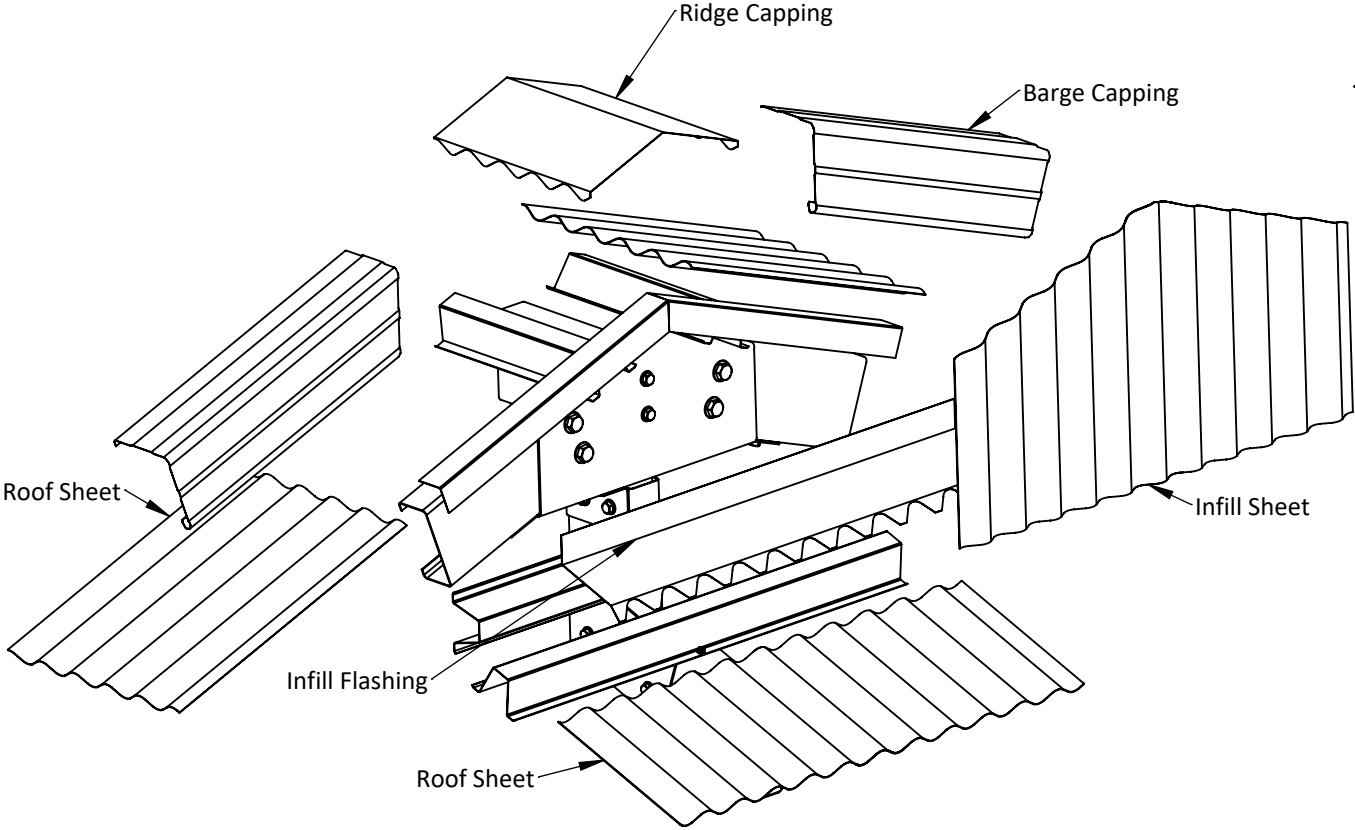
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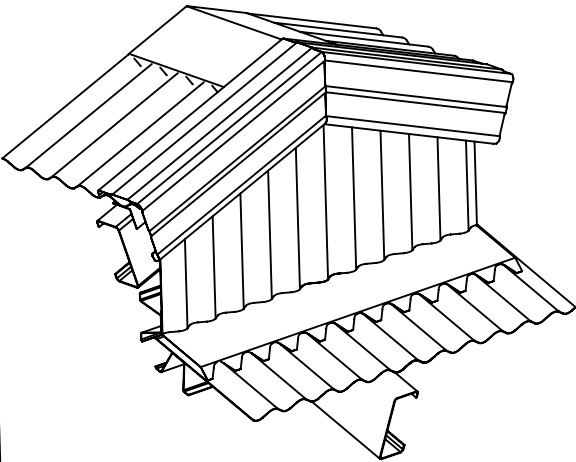
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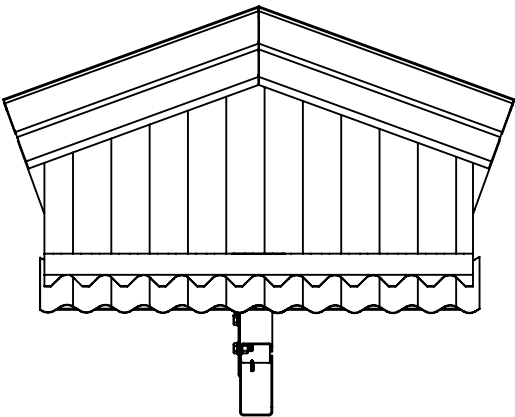
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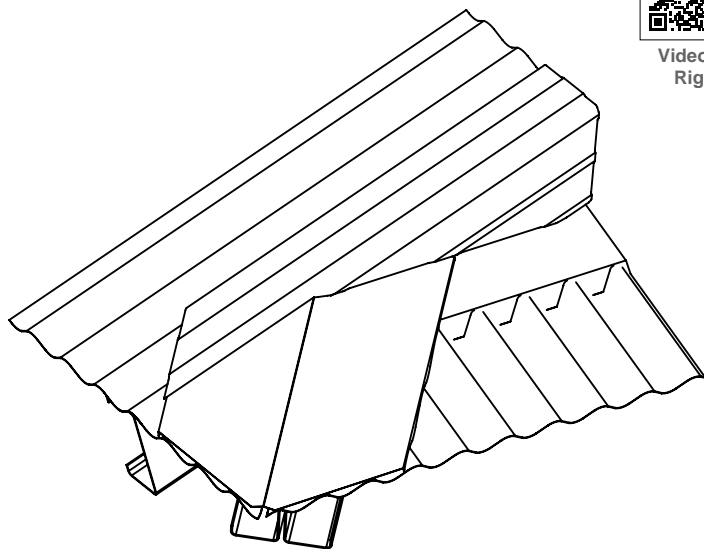
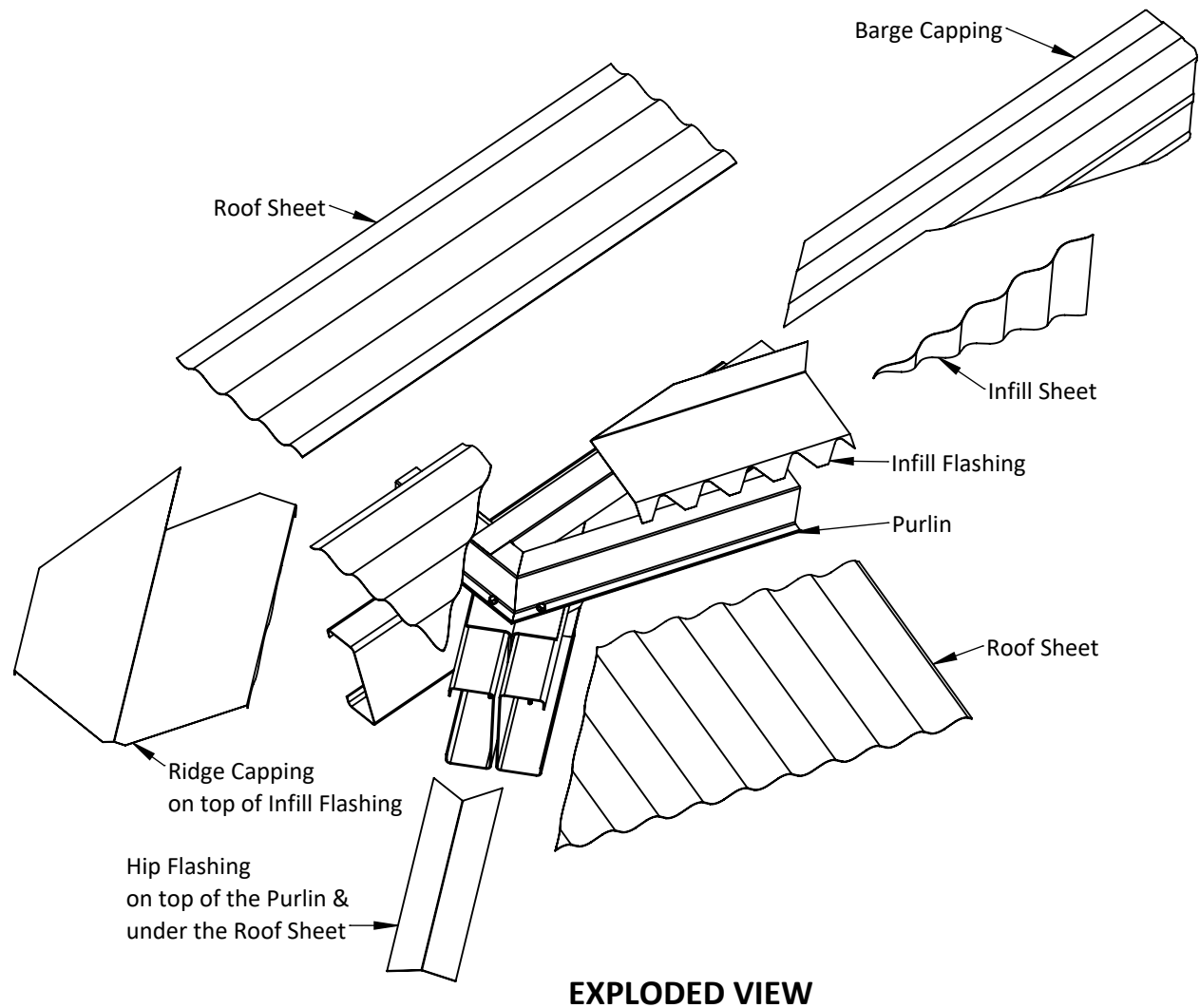
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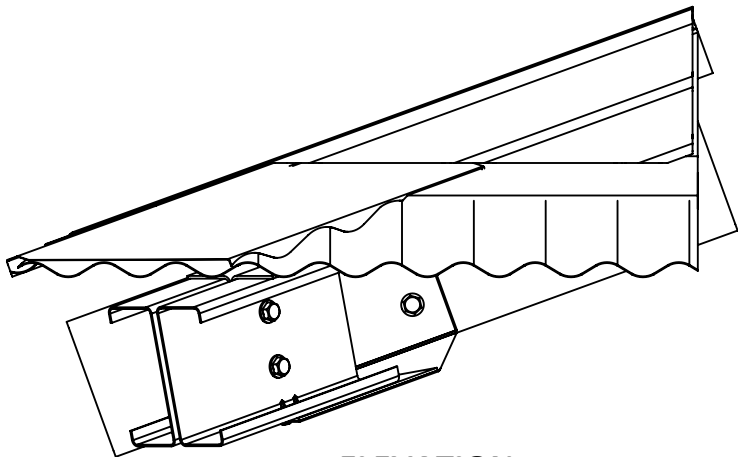
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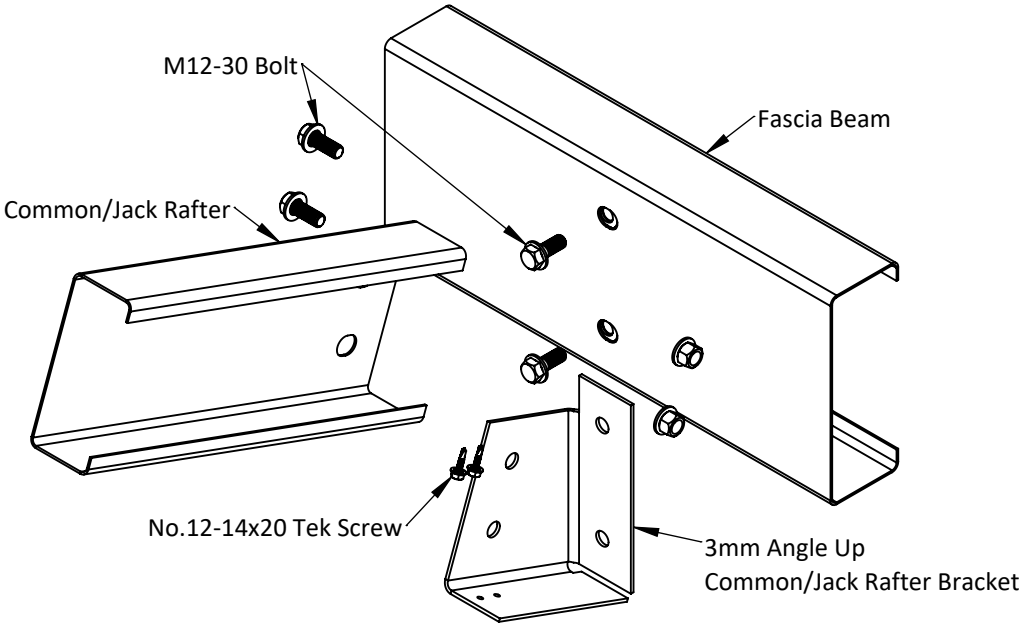


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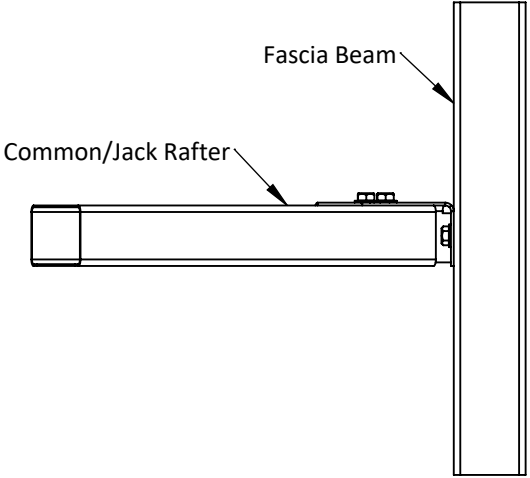
« Detail HC2.5: Common or Jack Rafter to Fascia Beam Connection Detail



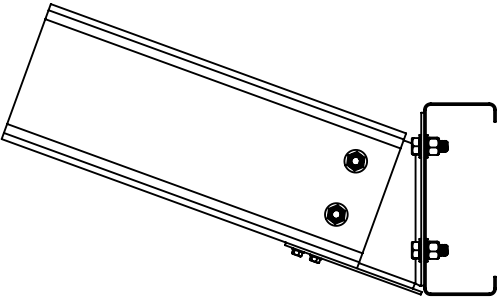
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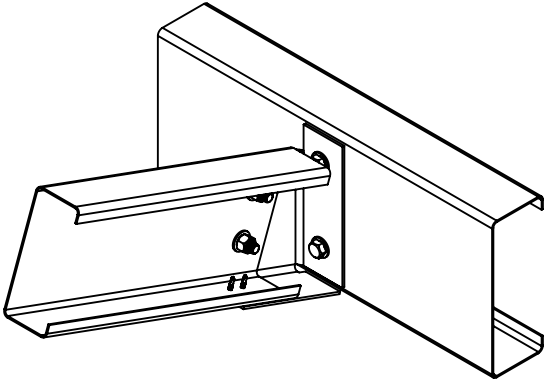
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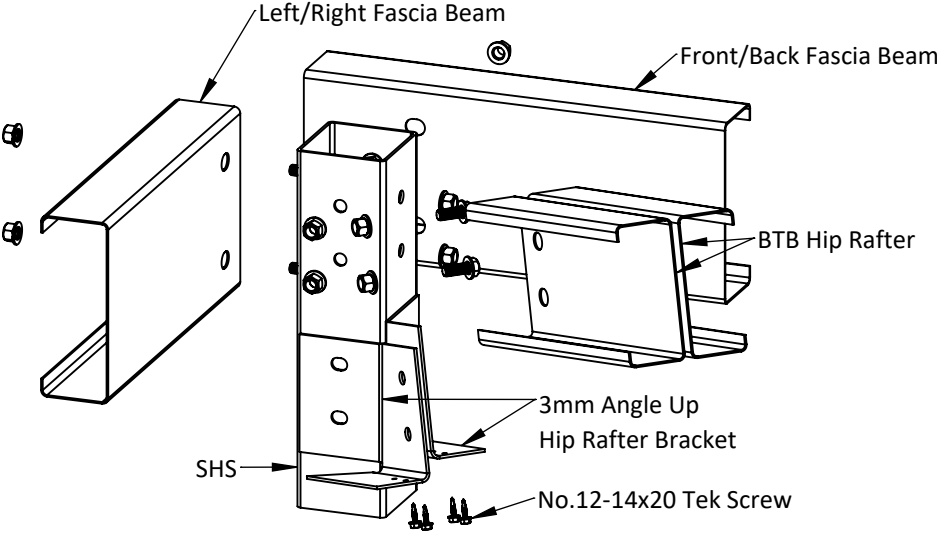
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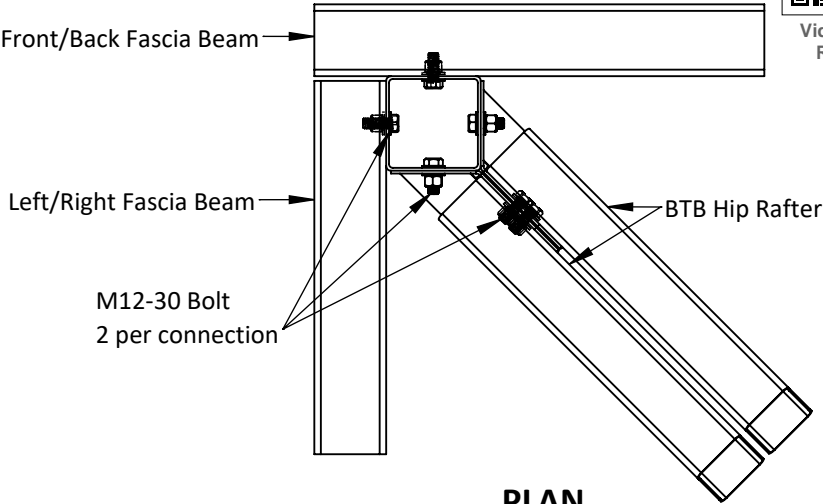
« Detail HC2.6: Hip Rafter to Post Connection Detail



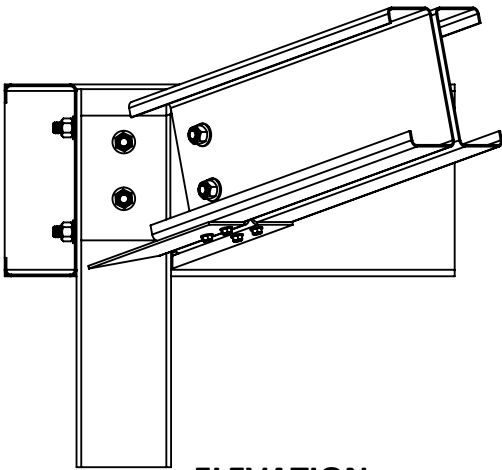
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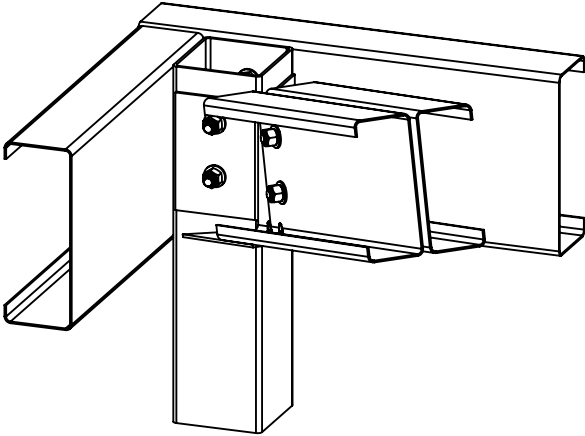
EXPLODED VIEW



PLAN



ELEVATION



ISOMETRIC

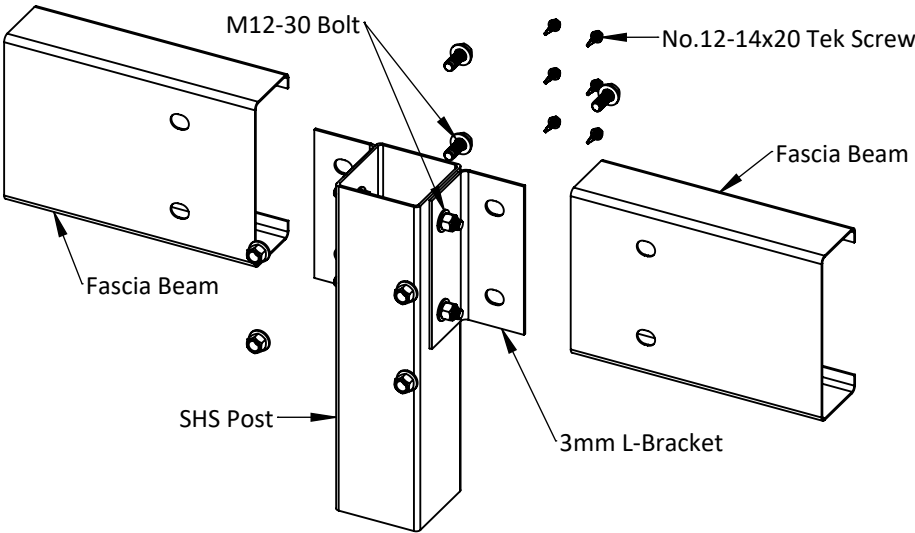


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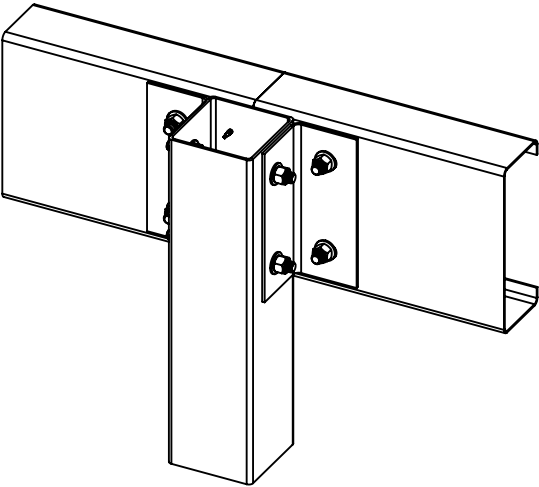
« Detail HC2.7: Fascia Beams to Post Connection Detail



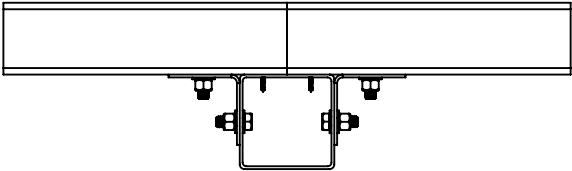
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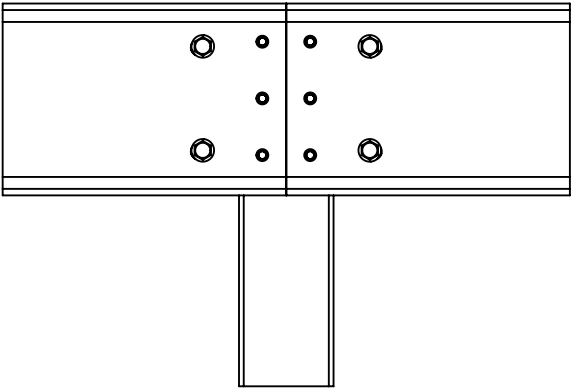
EXPLODED VIEW



ISOMETRIC



PLAN



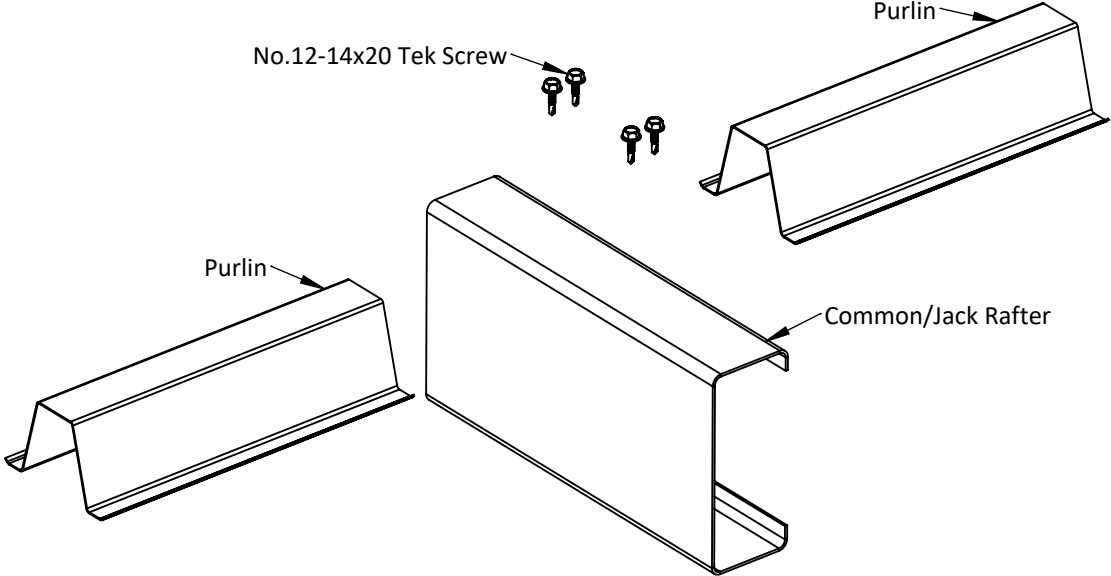
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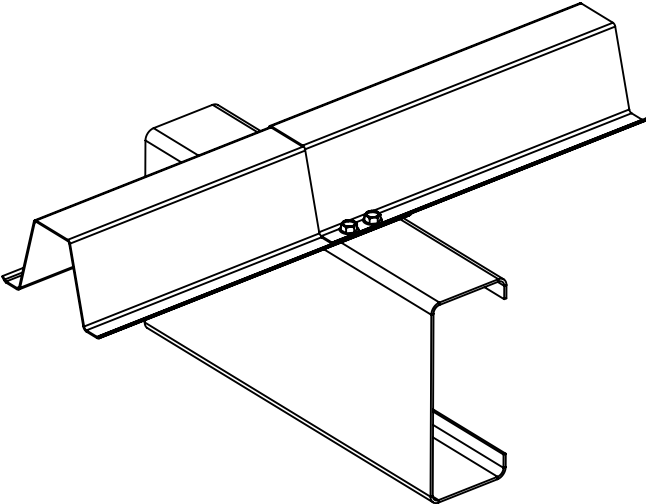
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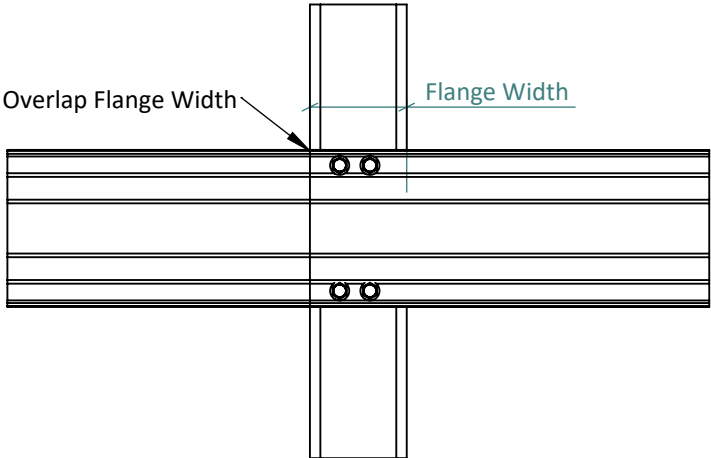
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EXPLODED VIEW



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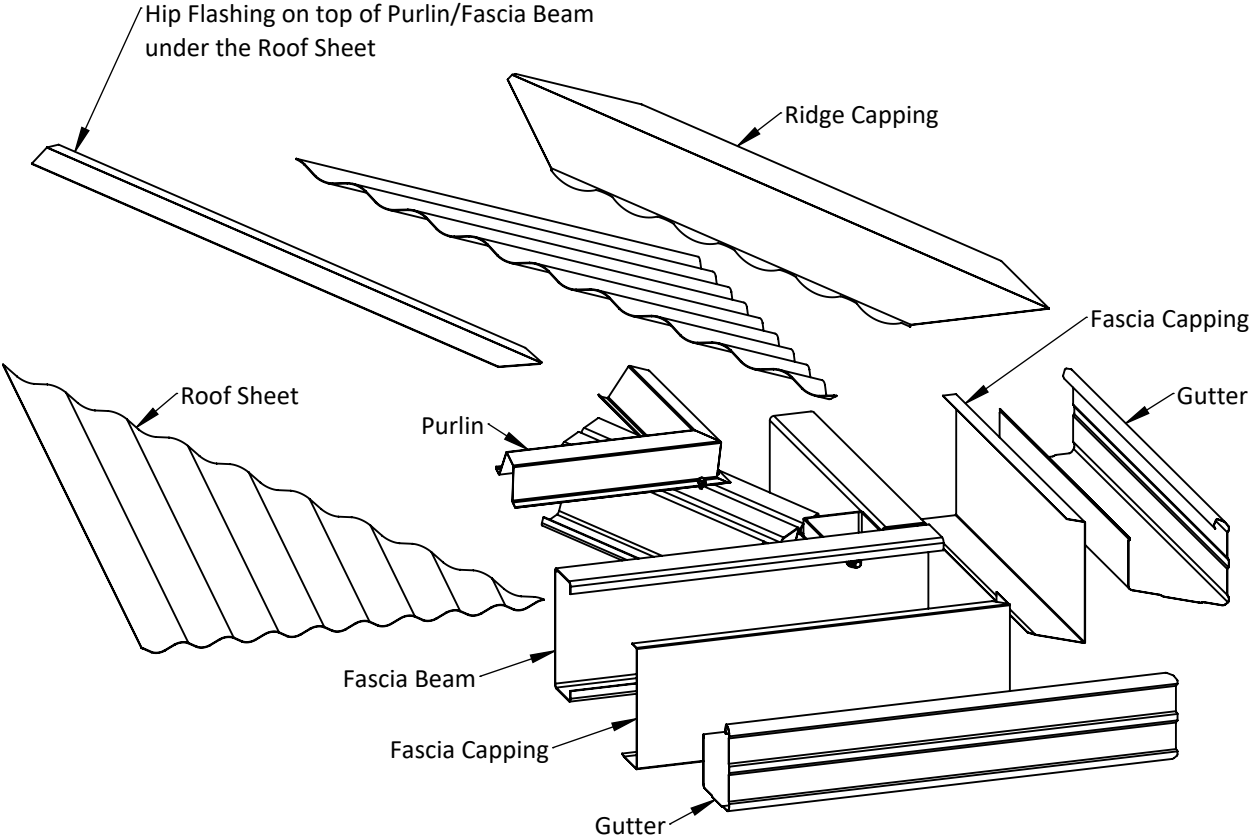
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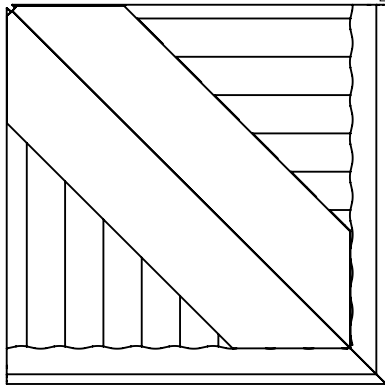
« Detail HC4.5.1: Cladding Connection Detail



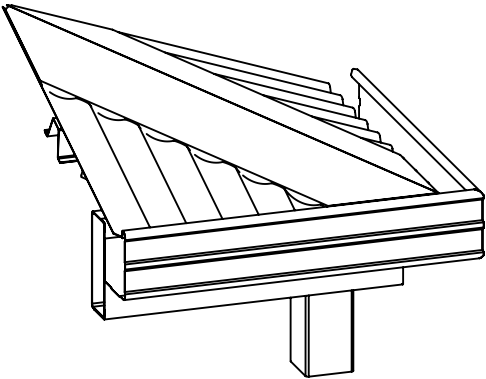
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EXPLODED VIEW



PLAN



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PROJECT:
Example 7000 x 12000 x 2400 Region B1 Dutch Gable Carport

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PROJECT NUMBER:
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<div>Technical Data</div> <div>Region Terrain Category Shielding V_{design} Q_u Inward Load Outward Load</div> <div>B1 · Regional Wind Speed 57 m/s 2.5 No 47.11 m/s 1.332 kPa 0.826 kPa · max(0.495, 0.826, 0.12) 0.616 kPa · max(0.31, 0.616)</div>		<div>Member L Compression40.123 kN · L_{ex} 1390 · L_{ey} 1390 · L_{ez} 1390</div> <div>Common Rafter · CR · Calculations</div> <div>Details · Member Min Yield Stress · Grade · I_x Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity</div> <div>ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection</div> <div>C15024 with 1 row of Bridging 450 MPa · G450 · 2.54 x 10⁶mm⁴ 6680 mm · 703 mm 0.581 kN/m 0.433 kN/m 7.520 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 10.264 kNm 12.434 kNm · L_{ex} 3395 · L_{ey} 931 · L_{ez} 1747 7.650 kNm 12.030 kNm · L_{ex} 3395 · L_{ey} 1747 · L_{ez} 1747 11.5 mm · 22.3 mm Permitted</div> <div>Internal Common Rafter · ICR · Calculations</div> <div>Details · Member Min Yield Stress · Grade · I_x Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity</div> <div>ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection</div> <div>C15024 450 MPa · G450 · 2.54 x 10⁶mm⁴ 6680 mm · 1406 mm 1.161 kN/m 0.866 kN/m 4.090 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 6.476 kNm 12.241 kNm · L_{ex} 3395 · L_{ey} 931 · L_{ez} 3395 4.830 kNm 5.497 kNm · L_{ex} 3395 · L_{ey} 3395 · L_{ez} 3395 7.2 mm · 22.3 Permitted</div> <div>Jack Rafter · JR · Calculations</div> <div>Details · Member Min Yield Stress · Grade · I_x Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity</div> <div>ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection</div> <div>C15012 500 MPa · G500 · 1.29 x 10⁶mm⁴ 1646 mm · 1750 mm 1.446 kN/m 1.078 kN/m 8.460 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 0.490 kNm 5.310 kNm · L_{ex} 1646 · L_{ey} 841 · L_{ez} 1646 0.365 kNm 5.310 kNm · L_{ex} 1646 · L_{ey} 1646 · L_{ez} 1646 0.1 mm · 5.5 Permitted</div> <div>Governing Purlin Calculations</div> <div>Dutch End Purlin Min Yield Stress · Grade · I_x Gov Span · Load Width Capacity · Inward Req Inward Capacity Outward Req Outward Capacity</div> <div>TH6110 550 MPa · G550 · 0.16 x 10⁶mm⁴ 2630 mm · 841 mm 1.042 kN/m 1.550 kN/m · Single Span @ 3000mm Source: Lysaght® & Stramit® Span Tables 0.777 kN/m 1.380 kN/m · Single Span @ 3000 mm · Minimum Support Thickness of 1.2 mm BMT · 2 x No. 12 Tek Screws</div>		<div>Deflection Non-Dutch End Purlin Min Yield Stress · Grade · I_x Gov Span · Load Width Capacity · Inward Req Inward Capacity Outward Req Outward Capacity</div> <div>0.1 mm · 4.6 mm Permitted</div> <div>Dutch End Fascia Beam Calculations</div> <div>Member Min Yield Stress · Grade · I_x Gov Span Concentrated Load Capacity · Inward Req Outward Req Member Capacity</div> <div>ØM · Inward Req Outward Req ØM Deflection</div> <div>C20015 450 MPa · G450 · 3.53 x 10⁶mm⁴ 6748 mm One Point Load at 3374 mm 0.399 kN/m 0.298 kN/m 1.010 kN/m · 1 row of Bridging · Span @ 6900 mm Source: Lysaght® and Stramit® Span Tables 2.271 kNm 1.694 kNm 6.487 kNm · L_{ex} 6748 · L_{ey} 3374 · L_{ez} 3374 2.6 mm · 22.5 mm Permitted</div> <div>Fascia Beam Calculations for FB1.1 & FB1.2</div> <div>Details · Member Min Yield Stress · Grade · I_x Gov Span Concentrated Loads Capacity · Inward Req Outward Req Member Capacity</div> <div>ØM · Inward Req Outward Req ØM Deflection</div> <div>C20019 with 2 rows of Bridging 450 MPa · G450 · 4.51 x 10⁶mm⁴ 3784 mm 2 Point Loads at 1656 & 3062 mm 3.004 kN/m 2.239 kN/m 6.660 kN/m · 2 rows of Bridging · Span @ 3900 mm Source: Lysaght® & Stramit® Span Tables 5.377 kNm 4.008 kNm 14.823 kNm · L_{ex} 3784 · L_{ey} 1656 · L_{ez} 1656 0.8 mm · 12.6 mm Permitted</div> <div>Fascia Beam Calculations for FB2.1 & FB2.2</div> <div>Details · Member Min Yield Stress · Grade · I_x Gov Span Concentrated Loads Capacity · Inward Req Outward Req Member Capacity</div> <div>ØM · Inward Req</div> <div>C20019 with 3 rows of Bridging 450 MPa · G450 · 4.51 x 10⁶mm⁴ 3820 mm 3 Point Loads at 504, 1910 & 3316 mm 3.102 kN/m 2.314 kN/m 6.660 kN/m · 3 rows of Bridging · Span @ 3900 mm Source: Lysaght® & Stramit® Span Tables 5.658 kNm</div>					
<div><div><div></div><div>Your Logo Here</div></div></div>		<div>PROJECT: Example 7000 x 12000 x 2400 Region B1 Dutch Gable Carport</div> <div>CLIENT: Example Customer</div> <div>ADDRESS: Example Street, Suburb VIC 3333</div>		<div>PROJECT NUMBER: 6457</div> <div>DRAWN BY: dm3 Solutions</div> <div>DRAWN DATE: 2022-09-28</div>		<div>© 2022 dm3 Solutions. The Information contained herein is Proprietary, Confidential and the Sole Property of dm3 Solutions. Reproduction in part or in whole is Prohibited without written approval. Unless specified otherwise, dimensions are in millimetres & drawings are not to scale.</div> <div>Powered by dm3Solutions.com</div> <div>ISSUE: 1</div> <div>SIZE: A4</div> <div>SCALE: NTS</div> <div>DRAWING NUMBER 25 of 26</div>			



<div>Outward Req 4.22 kNm ØM 14.823 kNm · L_{ex} 3820 · L_{ey} 1406 · L_{ez} 1406 Deflection 1.1 mm · 12.7 mm Permitted</div>		<div>Capacity Req 5.684 kN Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity Connection Type FB2.1 & FB2.2 to Post Components 2 x M12 4.6/S Bolts Capacity Req 5.925 kN Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity Connection Type FB3.1 & FB3.2 to Post Components 2 x M12 4.6/S Bolts Capacity Req 8.470 kN Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity 1 x M12 4.6/S Bolt Web Bearing Capacity 24.883 kN · 1.5 mm Gauge C-Section 31.519 kN · 1.9 mm Gauge C-Section 39.813 kN · 2.4 mm Gauge C-Section Plate Bearing Capacity 49.766 kN · 3.0 mm Gauge Plate Web Bearing Capacity 53.084 kN · 2.4 mm Gauge C-Section 66.355 kN · 3.0 mm Gauge C-Section Plate Bearing Capacity 66.355 kN · 3.0 mm Gauge Plate Tear Out Capacities Plates Minimum · Top Bolt 51.840 kN Minimum · Bottom Bolt 77.760 kN</div>					
<div>Fascia Beam Calculations for FB3.1 & FB3.2</div>							
<div>Details · Member C20019 with 2 rows of Bridging Min Yield Stress · Grade · I_x 450 MPa · G450 · 4.51 x 10⁶mm⁴ Gov Span 3784 mm Concentrated Loads 2 Point Loads at 722 & 2128 mm Capacity · Inward Req 4.477 kN/m Outward Req 3.337 kN/m Member Capacity 6.660 kN/m · 2 rows of Bridging · Span @ 3900 mm Source: Lysaght® & Stramit® Span Tables ØM · Inward Req 8.013 kNm Outward Req 5.973 kNm ØM 14.823 kNm · L_{ex} 3784 · L_{ey} 1656 · L_{ez} 1656 Deflection 0.6 mm · 12.6 mm Permitted</div>							
<div>Cladding Details</div>							
<div>Product BlueScope® Colorbond® Min Yield Stress · Grade 550 MPa · G550 Profile CustomOrb® BMT 0.42 mm</div>							
<div>End Posts Bored Piers</div>							
<div>ØM · M_u 11.03 kNm Uplift · V_u 4.589 kN Shear · H_u 4.596 kN Dimensions 350 mm diameter x 1100 mm deep</div>							
<div>Internal Posts Bored Piers</div>							
<div>ØM · M_u 22.058 kNm Uplift · V_u 9.178 kN Shear · H_u 9.191 kN Dimensions 450 mm diameter x 1300 mm deep</div>							
<div>Connection Calculations</div>							
<div>Connection Type Hip Rafter to Post Components 2 x M12 4.6/S Bolts Capacity Req 3.041 kN Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity Connection Type Common Rafter to Fascia Components 2 x M12 4.6/S Bolts Capacity Req 5.577 kN Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity Connection Type Internal Common Rafter to Fascia Components 2 x M12 4.6/S Bolts Capacity Req 3.878 kN Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity Connection Type L/R Fascia Beam to Post Components 2 x M12 4.6/S Bolts Capacity Req 1.346 kN Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity Connection Type FB1.1 & FB1.2 to Post Components 2 x M12 4.6/S Bolts</div>							

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