	General Notes		Steel Grades	
G01	The Design and Details shown in these Drawings are applicable to this Project only.	300 PLUS	Universal Beams & Columns, Parallel Flange Channels Angles to AS/NZS3679.1	
G02	These Drawings shall be read in conjunction with all Architectural Drawings, other Consultants' Drawings, Specifications and such other Written	250	Flats, Small Angles, Taper Flange Beams & Columns t AS/NZS3679.1	
	Instructions as may be issued during the course of the Project. Any discrepancy shall be referred to the Engineer before proceeding with the work.	300	Welded Sections to AS/NZS3679.2	
G03	All Materials and Workmanship shall be in accordance with the relevant and	250	Hot Rolled Plates, Floor Plates & Slabs to AS/NZS3678	
003	current SAA Codes and Authorities Except where varied by the Project Specifications.	250	Hollow Sections to AS1163. Circular Sections less that Outside Diameter. Sections other than the above	
G04	The Structure must be maintained in a stable condition and no part must be overloaded during construction. Temporary bracing must be designed and	G450-G550	Cold Form AS4600. Unless noted otherwise, all purlins brackets are G450.	
	provided by the contractor(s) to keep the building works and excavations	G500	Slab mesh and deformed reinforcement bars.	
	stable at all times.	G2070	AS2841. Galvanized Steel Wire Strand. Unless noted	
G05	The Issuer of these Designs reserves the right to alter Specifications and		all Cable Bracing is G2070.	
G06	Designs as it may see fit without prior notification or penalty. The Structure has not been designed for snow loads.	Australian Standard [™] & Australian/New Zealand Stand		
G07	Connections may require on-site drilling by the contractor(s).	AS 1111.1	ISO Metric Hexagon Bolts and Screws – Product Grad- Bolts	
	Foundation Notes	AS/NZS 1163	Cold-formed Structural Steel Hollow Sections	
F01	Foundations have been designed assuming an allowable bearing capacity of	AS/NZS 1170.0	Structural Design Actions - Part 0: General Principles	
	100 kPa.	AS/NZS 1170.1	Structural Design Actions - Part 1: Permanent, Impose	
F02	The Owner is advised to obtain a Geotechnical Report and/or Site Foundation		Actions	
F00	Report to confirm F01 above.	AS/NZS 1170.2	Structural Design Actions - Part 2: Structural Design, V	
F03	Footings shall be located centrally under walls and columns unless noted otherwise.	AS/NZS 1170.4	Structural Design Actions - Part 4: Earthquake Actions	
F04	All Footings to be founded a minimum of 400 mm into Natural Ground. Do not	AS 1252	Structural Assemblies	
104	Found Footings in uncontrolled fill.	AS/NZS 1554	Structural Steel Welding	
F05	Engineer to be contacted if Foundation Conditions vary from F01 above.	AS 4100	Steel Structures	
	Steelwork Notes	AS/NZS 4600	Cold-formed Steel Structures	
S01	All Workmanship and Materials shall be in accordance with AS 4100, AS/NZ	AS/NZS 4671	Steel Reinforcing Materials	
001	4600 and AS 1554 except where varied by the Project documentation.	AS/NZS 4680	Hot-dip Galvanized (Zinc) Coatings on Fabricated Ferr	
S02	Bolting Categories are identified on the Structural Drawings as follows:		Region & Site Details	

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This is a freestanding roof only structure. It is assumed that cars, goods or materials stored under the roof block less than 50 percentage of the cross section exposed to the wind.

Suitable for Regions A1-5 & B1. TC2.5 without Shielding: Terrain with some trees or isolated obstructions, terrain in developing outer urban areas with scattered houses, or large acreage developments with more than two and less than 10 buildings per hectare. The site is not on the slope or crest of a hill, ridge or escarpment.

 $V_{design} = V_R \times M_c \times M_d \times (M_{z,Cat} \times M_s \times M_t)$

 $V_{design} = 57 \times 1 \times 0.95 \times (0.87 \times 1 \times 1) = 47.11 \text{ m/s}$

Foundations have been designed assuming an allowable bearing capacity of 100 kPa, undrained cohesion $C_u = 50$ kPa. Concrete Strength = 25 MPa.

G01	The Design and Details shown in these Drawings are applicable to this Project only.	300 PLUS	Universal Beams & Columns, Parallel Flange Channels, Large Angles to AS/NZS3679.1
G02	These Drawings shall be read in conjunction with all Architectural Drawings, other Consultants' Drawings, Specifications and such other Written	250	Flats, Small Angles, Taper Flange Beams & Columns to AS/NZS3679.1
	Instructions as may be issued during the course of the Project. Any discrepancy shall be referred to the Engineer before proceeding with the work.	300	Welded Sections to AS/NZS3679.2
G03	All Materials and Workmanship shall be in accordance with the relevant and	250	Hot Rolled Plates, Floor Plates & Slabs to AS/NZS3678
G03	current SAA Codes and Authorities Except where varied by the Project Specifications.	250	Hollow Sections to AS1163. Circular Sections less than 165mm Outside Diameter. Sections other than the above
G04	The Structure must be maintained in a stable condition and no part must be overloaded during construction. Temporary bracing must be designed and	G450-G550	Cold Form AS4600. Unless noted otherwise, all purlins, plates & brackets are G450.
	provided by the contractor(s) to keep the building works and excavations	G500	Slab mesh and deformed reinforcement bars.
	stable at all times.	G2070	AS2841. Galvanized Steel Wire Strand. Unless noted otherwise,
G05	The Issuer of these Designs reserves the right to alter Specifications and Designs as it may see fit without prior notification or penalty.		all Cable Bracing is G2070.
G06	The Structure has not been designed for snow loads.	Australian	Standard [™] & Australian/New Zealand Standard [™] Adopted
G07	Connections may require on-site drilling by the contractor(s).	AS 1111.1	ISO Metric Hexagon Bolts and Screws – Product Grade C Part 1: Bolts
	Foundation Notes	AS/NZS 1163	Cold-formed Structural Steel Hollow Sections
F01	Foundations have been designed assuming an allowable bearing capacity of	AS/NZS 1170.0	Structural Design Actions - Part 0: General Principles
F00	100 kPa.	AS/NZS 1170.1	Structural Design Actions - Part 1: Permanent, Imposed & Other
F02	The Owner is advised to obtain a Geotechnical Report and/or Site Foundation Report to confirm F01 above.	AS/NZS 1170.2	Actions Structural Design Actions - Part 2: Structural Design, Wind Actions
F03	Footings shall be located centrally under walls and columns unless noted	AS/NZS 1170.2 AS/NZS 1170.4	3 ,
	otherwise.	AS/NZS 1170.4 AS 1252	Structural Design Actions - Part 4: Earthquake Actions in Australia Structural Assemblies
F04	All Footings to be founded a minimum of 400 mm into Natural Ground. Do not	AS 1252 AS/NZS 1554	
	Found Footings in uncontrolled fill.	AS/NZS 1554 AS 4100	Structural Steel Welding Steel Structures
F05	Engineer to be contacted if Foundation Conditions vary from F01 above.	AS/NZS 4600	Cold-formed Steel Structures
	Steelwork Notes	AS/NZS 4600 AS/NZS 4671	
S01	All Workmanship and Materials shall be in accordance with AS 4100, AS/NZ	AS/NZS 4671 AS/NZS 4680	Steel Reinforcing Materials Hot-dip Galvanized (Zinc) Coatings on Fabricated Ferrous Articles
	4600 and AS 1554 except where varied by the Project documentation.	A3/NZ3 4000	
202	Politing Catagorias are identified on the Structural Drawings as follows:		Pagion 9 Sita Dataila

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Bars to AS 4671 at 150 mm pitch.

less than 2 bolts.

Standard.

4.6/S Commercial Bolts of Grade 4.6 to AS 1111 Snug Tightened

S03 Unless noted otherwise, all M16 fasteners shall be Category 8.8/S. Standard

S04 All metal cladding should comply with AS/NZS 4680 G550, AZ150 (550 MPa

concrete shall be Grade N25 unless noted otherwise providing a cover adequate to suit fire rating or exposure conditions. Concrete Encasement shall

be centrally reinforced with 5 mm wire to AS 4671 or 6 mm Structural Grade

S05 Steelwork intended to be concrete encased shall be unpainted. Encasing

S06 Unless noted otherwise, all steel shall comply with the relevant Australian

minimum yield stress, 150 g/m2 minimum coating mass).

8.8/S High Strength Structural Bolts of Grade 8.8 to AS 1252 Snug Tightened

Fixings for C100, C150, C200 and C250 Sections are M12 bolts. Standard

Fixings for C300 and C350 Sections are M16 bolts. No Connection shall have

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Design Notes & Structural Members

7000 x 12000 x 2400

See Rafter Layout

TH6110 G550

3.0 mm G450

Cast In

Page 25

See Post & Footing Layout

See Fascia Beam Layout

47.11 m/s

Dimensions **Building Class** Design Wind Speed

Columns C

Rafters R

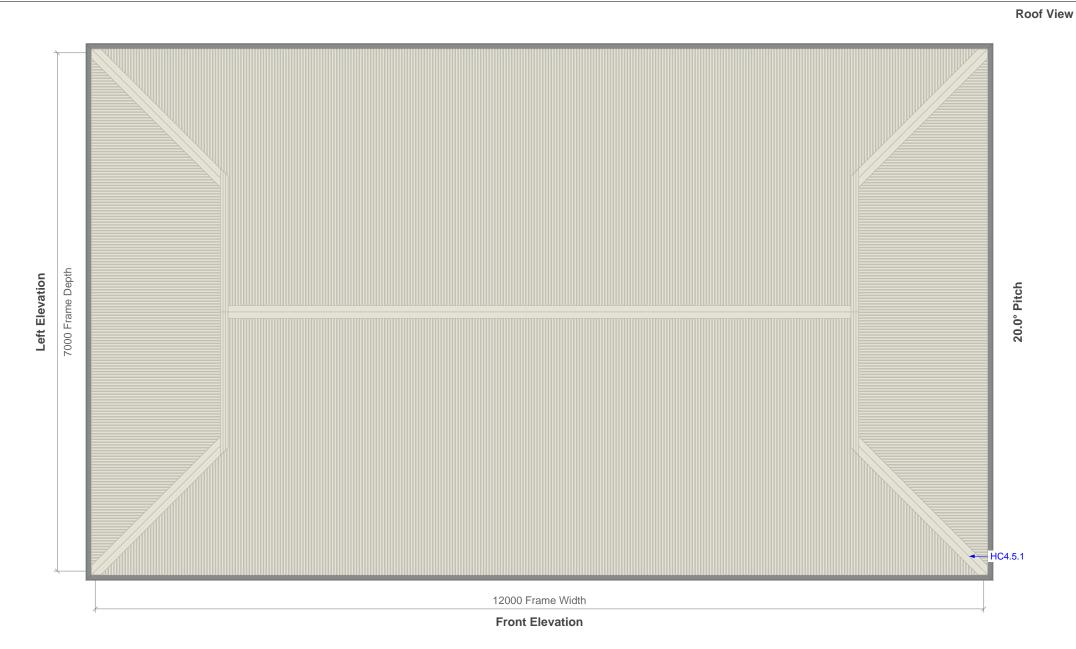
Purlins P

Footings

All Brackets

Technical Data

Fascia Beam FB



Note: Roof structure is light foot trafficable for maintenance purposes.



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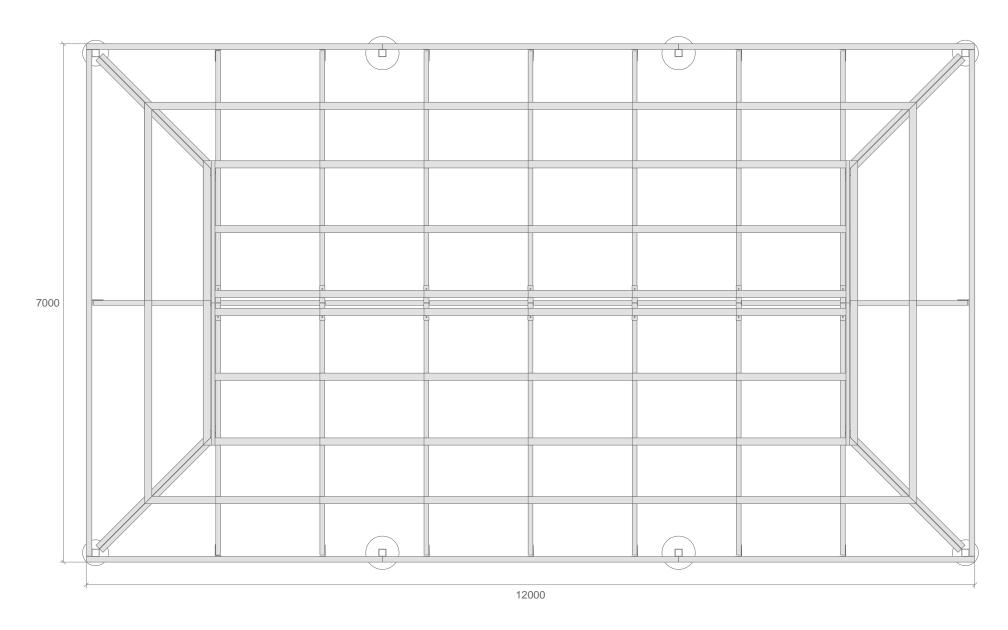
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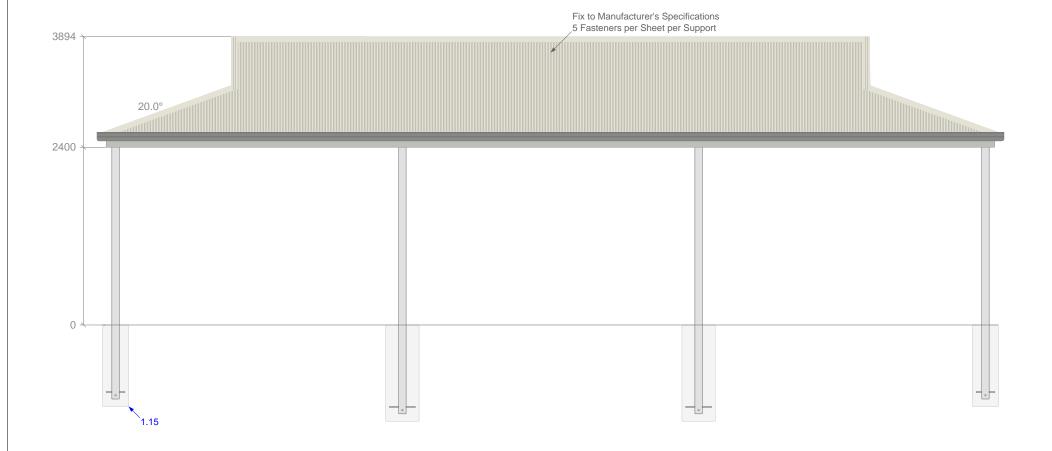
Plan View



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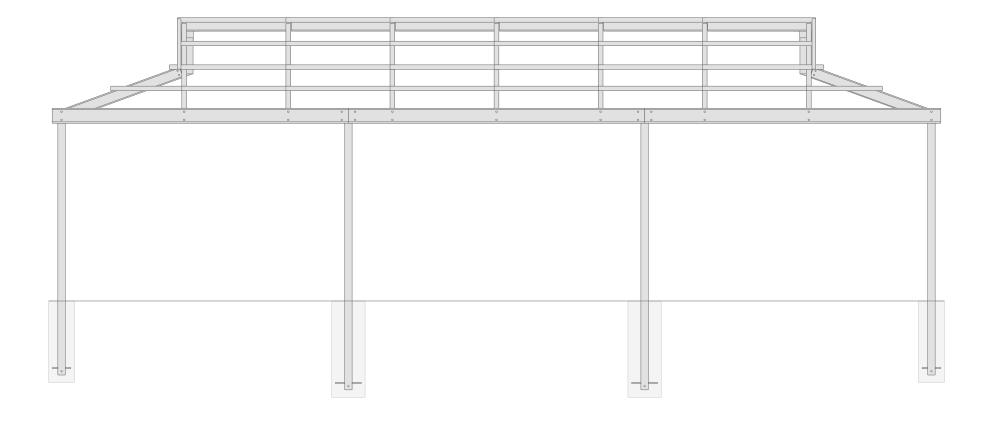
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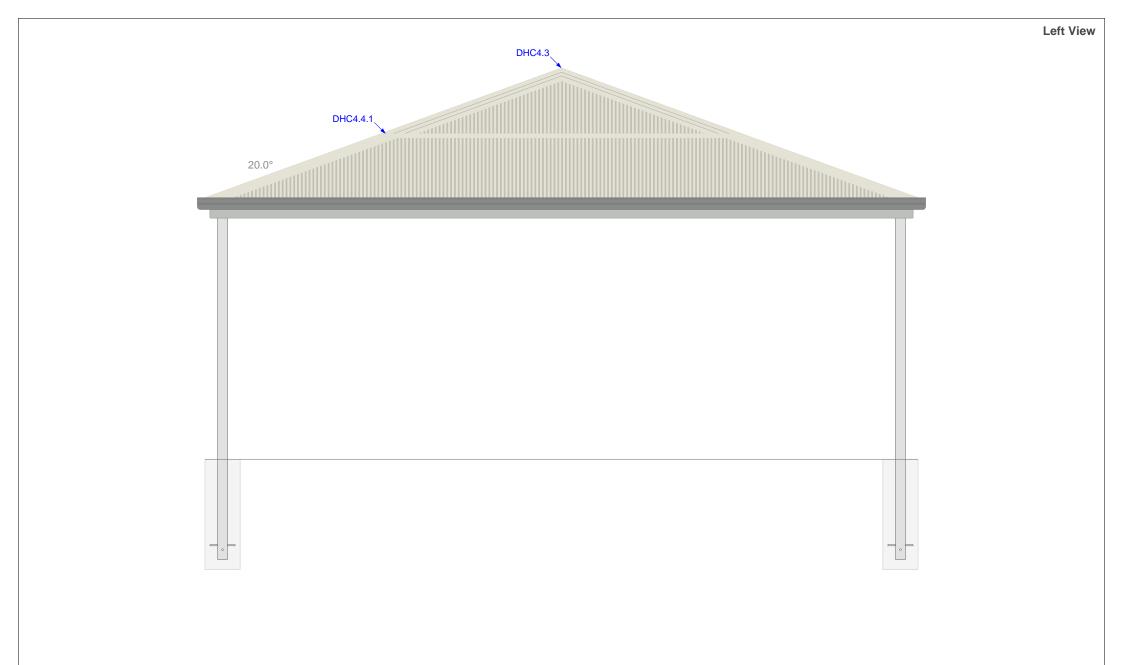


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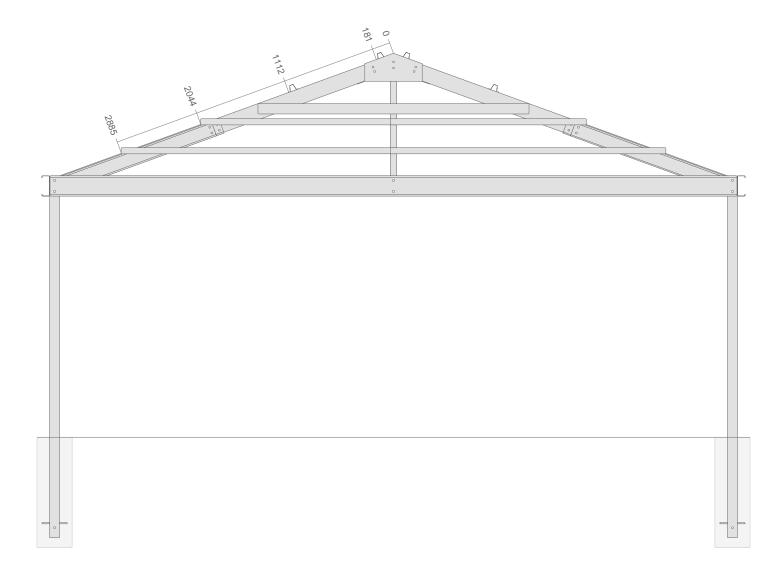
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Left Elevation



Dutch Gable Infill Members: TH6110 Infill Girt @ 2698 mm & C15012 Mullion @ 487 mm



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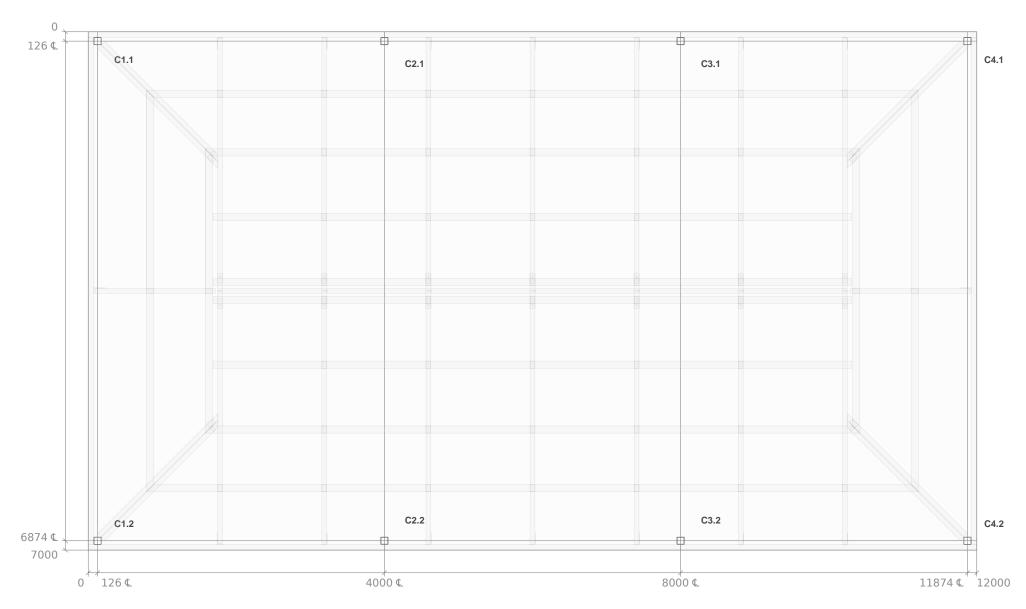
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Post & Footing Layout





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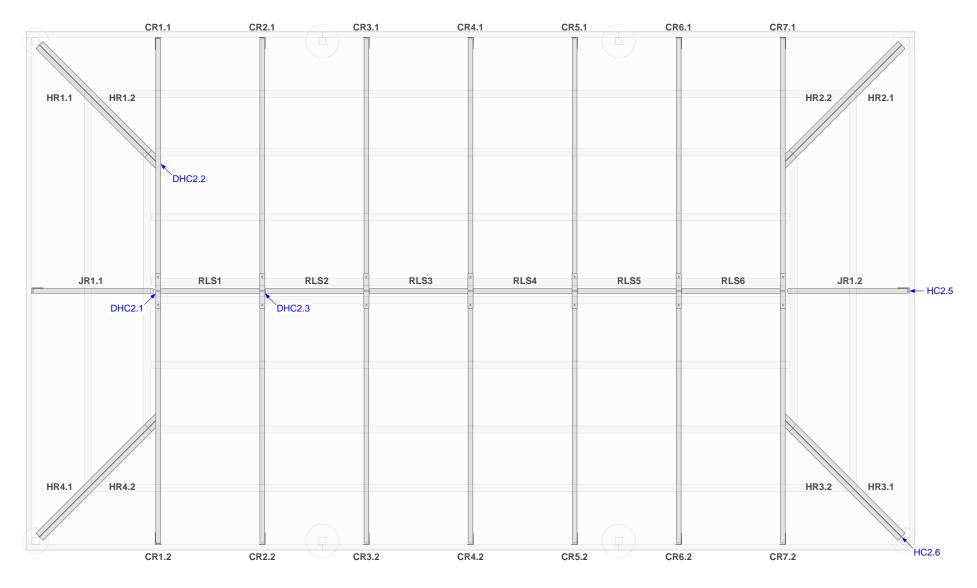
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Rafter Layout



CR = Common Rafter · C15024

JR = Jack Rafter · C15012

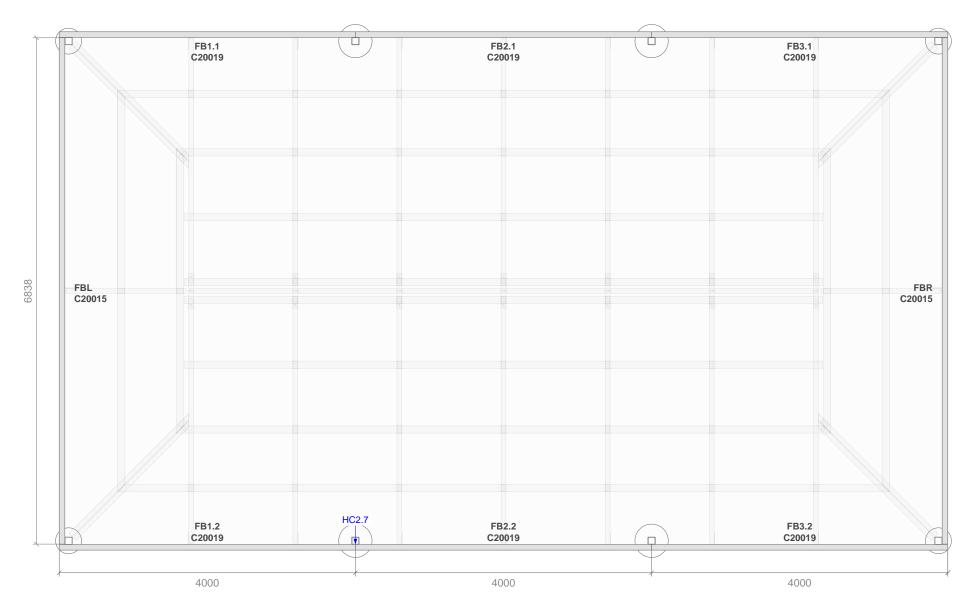
Note: All Rafter to Fascia Connections Use Detail HC2.5 Above

HR = Hip Rafter · C15012

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RLS = Ridge Line St	rut · C15012 Use Detail HC2.5 Above					
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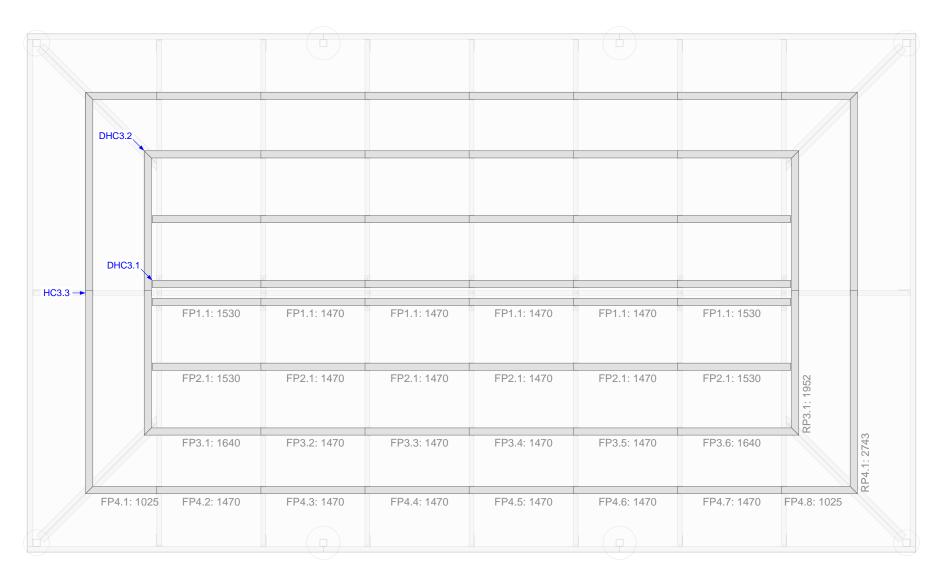
Fascia Layout



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Purlin Layout

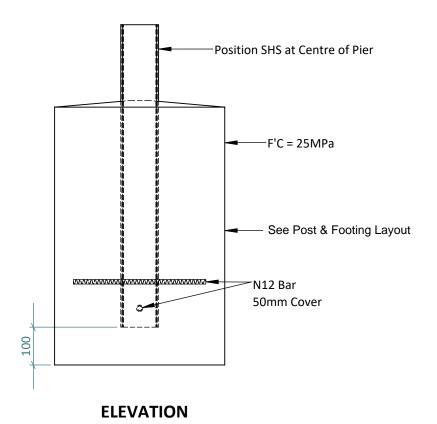


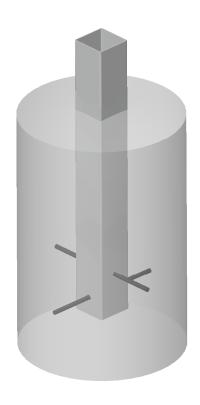


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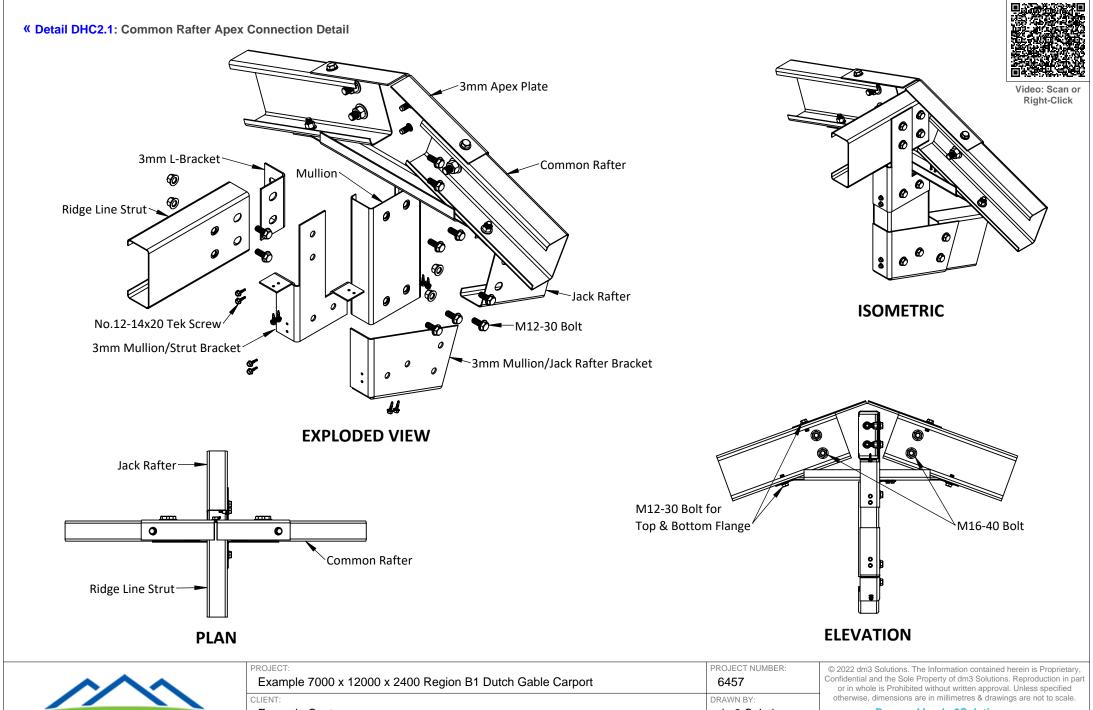




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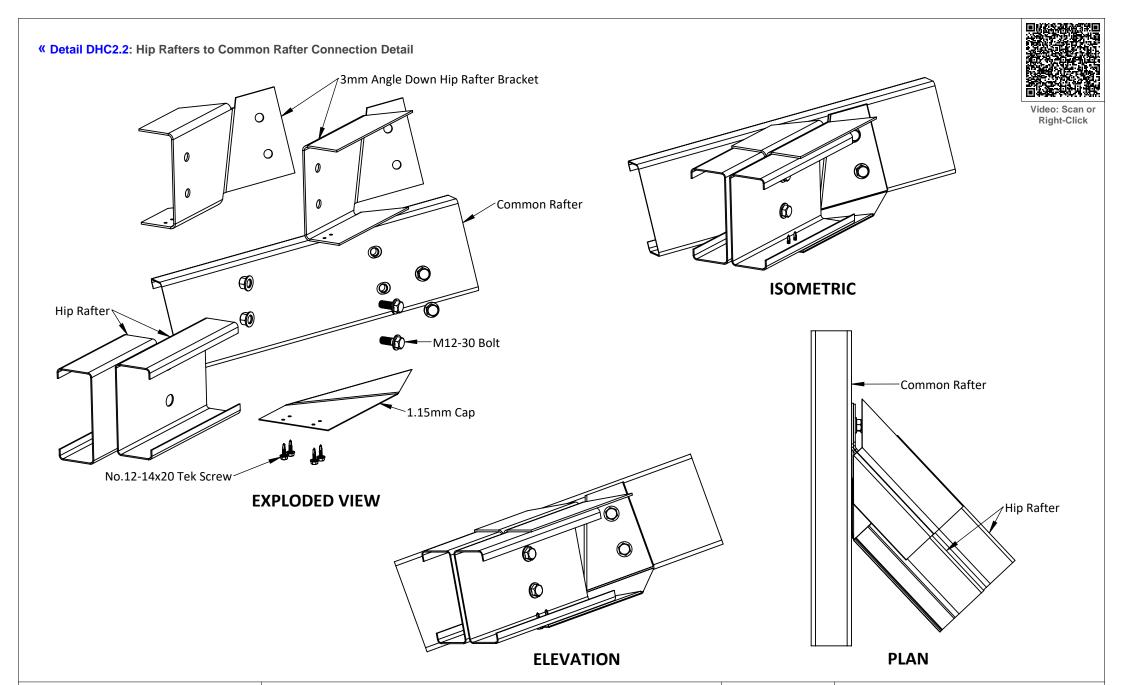
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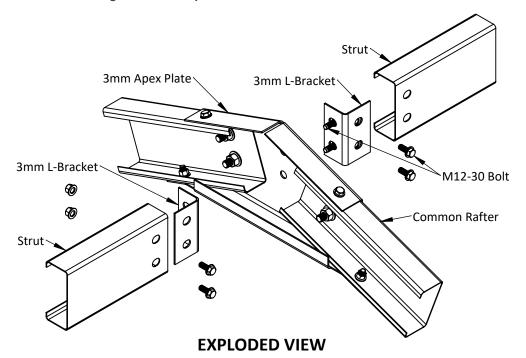
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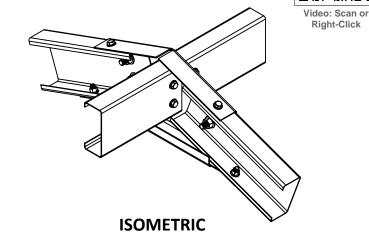
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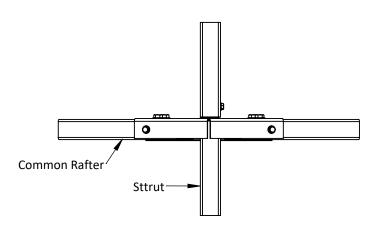
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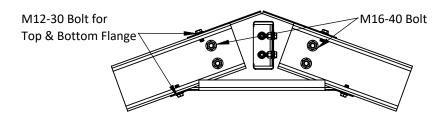
《 Detail DHC2.3: Ridge Line Strut Apex Connection Detail







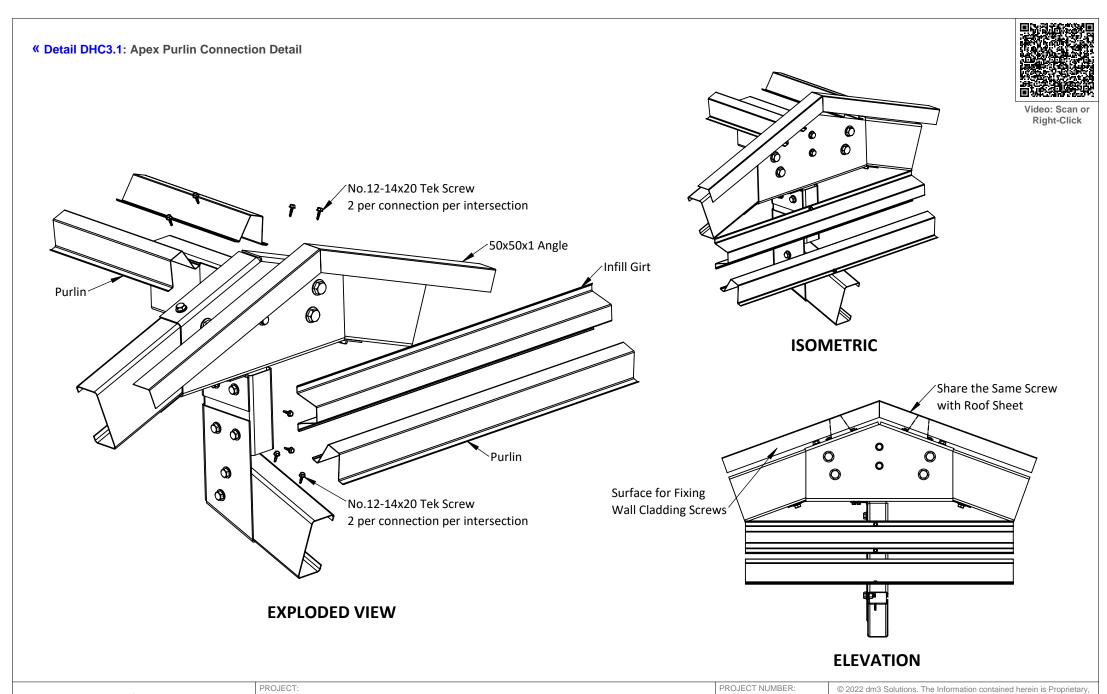
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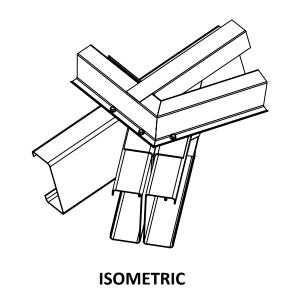
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No.12-14x20 Tek Screw

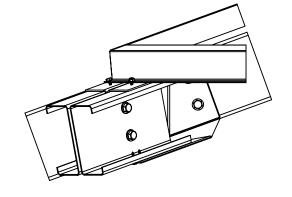
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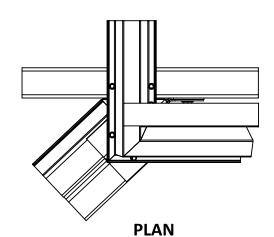




50x50x1 Angle

Purlin



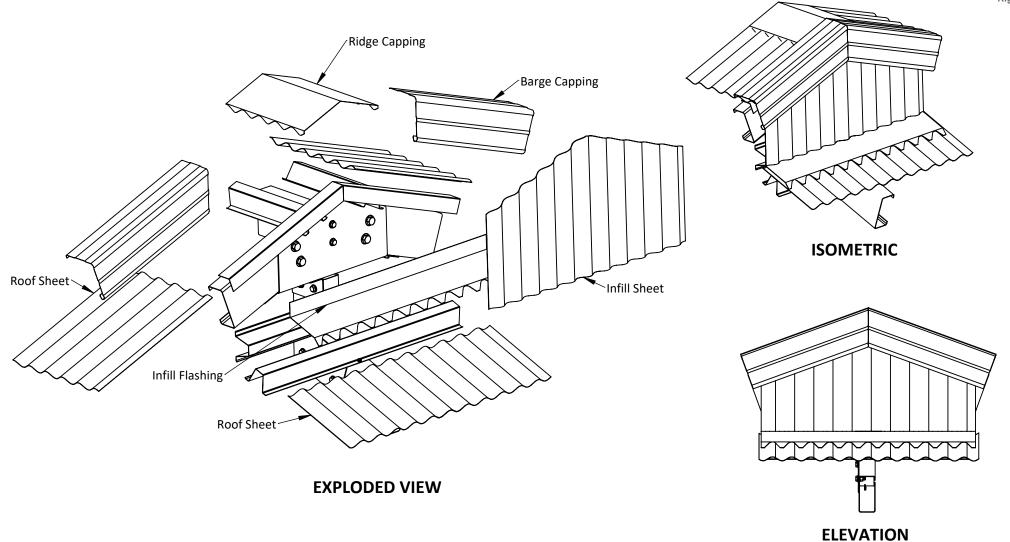


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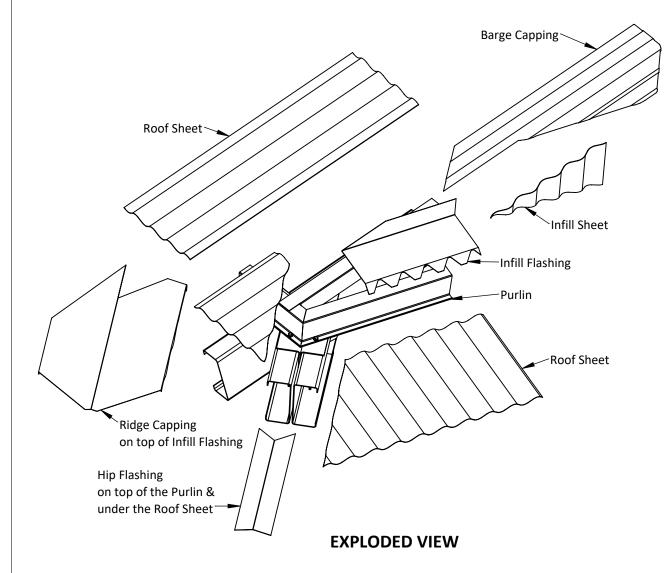
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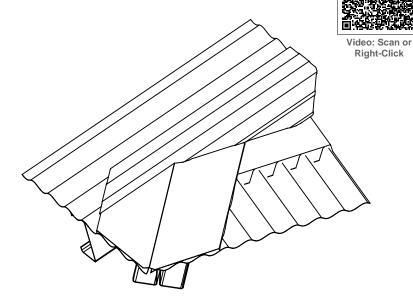




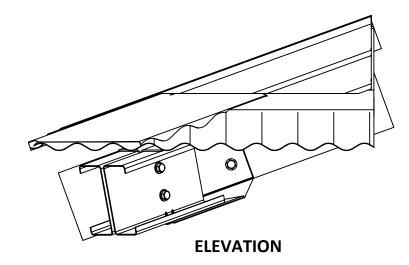
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« Detail DHC4.4.1: Infill Connection Detail





ISOMETRIC



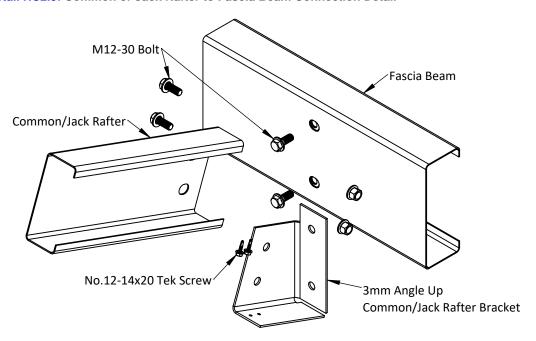


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《 Detail HC2.5: Common or Jack Rafter to Fascia Beam Connection Detail

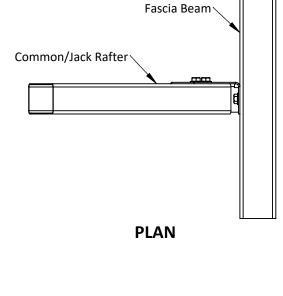


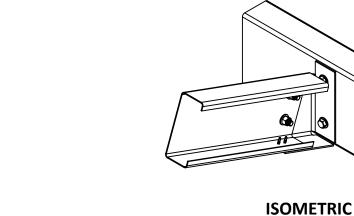
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ELEVATION



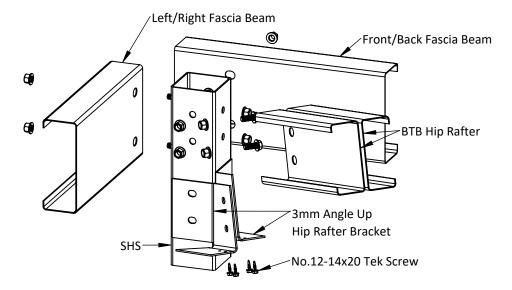


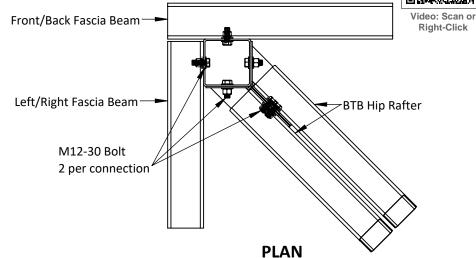


	PROJECT: Example 7000 x 12000 x 2400 Region B1 Dutch Gable Carport	PROJECT NUMBER: 6457	© 2022 dm3 Solutions. The Information contained herein is Proprietary, Confidential and the Sole Property of dm3 Solutions. Reproduction in part or in whole is Prohibited without written approval. Unless specified otherwise, dimensions are in millimetres & drawings are not to scale. Powered by dm3Solutions.com			
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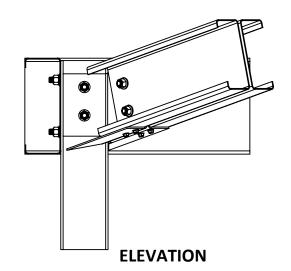
« Detail HC2.6: Hip Rafter to Post Connection Detail

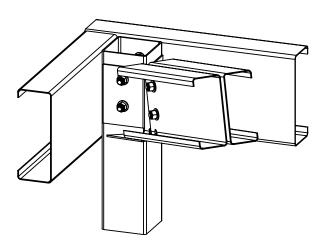






EXPLODED VIEW





ISOMETRIC



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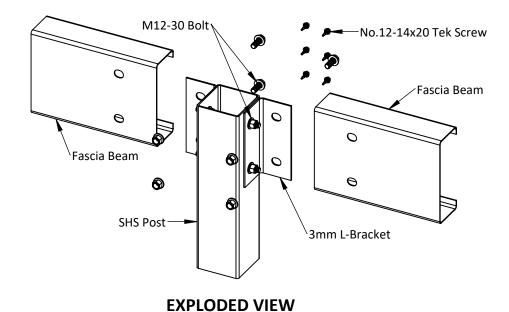
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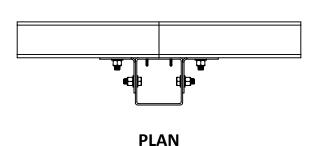
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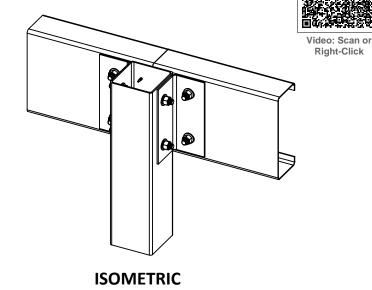
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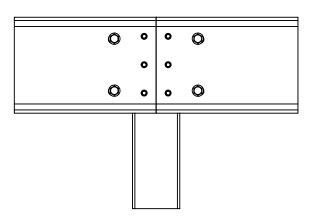
20	22-09-28	1	A4	NTS	21 of 26
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« Detail HC2.7: Fascia Beams to Post Connection Detail









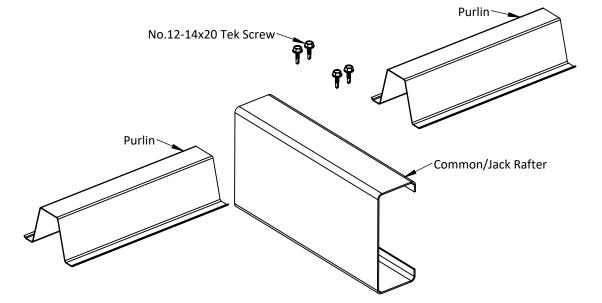
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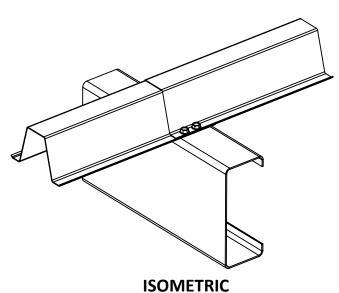
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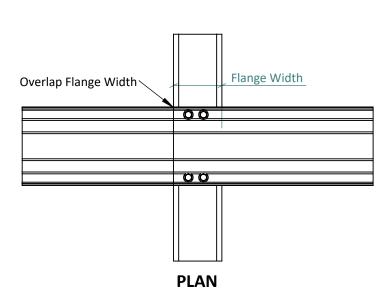
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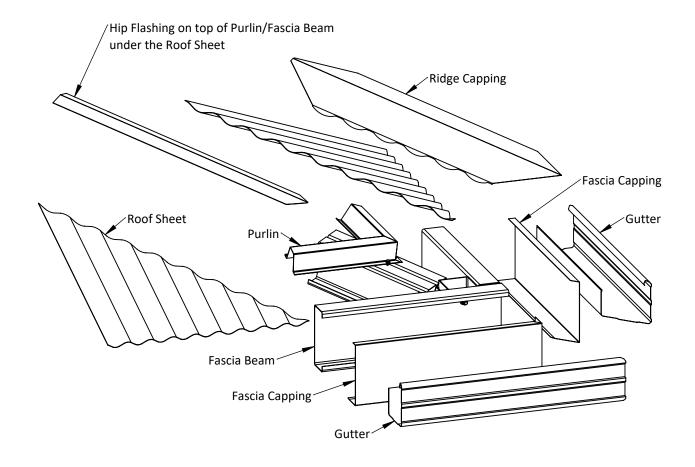
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« Detail HC4.5.1: Cladding Connection Detail



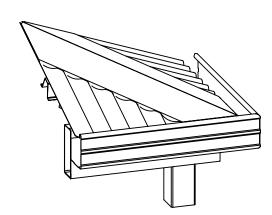


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PLAN





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2022-09-28	1	A4	NTS	24 of 26

	Technical Data		Member L Compression	40.123 kN · L _{ex} 1390 · L _{ey} 1390 · L _{ez} 1390			, ,		it [®] Span Tables
Region B1 · Regional Wind Speed 57 m/s		Common			1.7 mm · 8.8 mm Permitted				
Q _u Inward Load			Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Outward Req	0.581 kN/m 0.433 kN/m		Grade · I _x ad Width vard Req Capacity	550 MPa · G: 1376 mm · 9 1.154 kN/m 5.520 kN/m Source: Lysag	31 mm Single Span	
Roof, Rafte	er & Post Area Calcu	ulations	меттрег сарасіту	7.520 kN/m · min(Inward, Outward) Source: Lysaght [®] & Stramit [®] Span Tables			0.860 kN/m	Cinala Casa	0.1500
Fascia to Apex Dutch End Area Front Elevation Area Gable Infill Area Roof, Rafter & Post Area Calculations 3725 mm 9.775 m² 34.921 m² 1.115 m²		Outward Req Outward ⊘M	10.264 kNm 12.434 kNm \cdot L _{ex} 3395 \cdot L _{ey} 931 \cdot L _{ez} 1747 7.650 kNm 12.030 kNm \cdot L _{ex} 3395 \cdot L _{ey} 1747 \cdot L _{ez} 1747	Outward (3.630 kN/m · Single Span Minimum Support Thicknes 2 x No. 12 Tek Screws Source: Lysaght® & Strami 0.1 mm · 4.6 mm Permitte 		ess of 1.2 mm BMT \cdot	
	5.820 m ² · RF = Rak		Deflection 11.5 mm · 22.3 mm Permitted		Dutch End Fascia Beam Calculations				ns
Post Uplift Area Int Post Uplift Area	Int Post RF Load Area Post Uplift Area Int Post Uplift Area Int Post Uplift Area		$\begin{array}{c} \textbf{Details} \cdot \text{Member} \\ \text{Min Yield Stress} \cdot \text{Grade} \cdot I_x \end{array}$	450 MPa · G450 · 2.54 x 10 ⁶ mm ⁴	Min Yield Stress · Grade · I _x Gov Span		C20015 450 MPa · G450 · 3.53 x 10 ⁶ mm ⁴ 6748 mm		
	Post Calculations		Capacity · Inward Req	6680 mm · 1406 mm			One Point Lo	oad at 3374	l mm
Uplift · End Post · Inward Outward Int Post · Inward	450 MPa · Min = Min 6.153 kN 4.589 kN 12.306 kN	nimum	Outward Req Member Capacity Ø M · Inward Req	0.866 kN/m 4.090 kN/m · min(Inward, Outward) Source: Lysaght [®] & Stramit [®] Span Tables	Member (vard Req Capacity	0.298 kN/m 1.010 kN/m 6900 mm		idging · Span @ amit [®] Span Tables
RF · Front End Post · Req Front Int Post · Req Dutch End Post · Req	9.191 kN 2.337 kN	equired	Outward Req Outward ⊘M Deflection	4.830 kNm 5.497 kNm · L _{ex} 3395 · L _{ey} 3395 · L _{ez} 3395 7.2 mm · 22.3 Permitted	Outw	vard Req ⊘M	1.694 kNm		_{2y} 3374 · L _{ez} 3374 tted
ØM · End Post · Req 11.030 kNm ØM 13.900 kNm · 100x100x3 Source: OneSteel® DuraGal® Hollow Sections Int Post · ØM Req 22.058 kNm ØM 25.700 kNm · 100x100x5 Source: OneSteel® DuraGal® Hollow Sections Hip Rafter · HR · Calculations		·100··2	Jack R	Fascia Beam Calculations for FB1.1 & FB1.2					
		Gov Span · Load Width Capacity · Inward Req Outward Req	C15012 500 MPa · G500 · 1.29 x 10 ⁶ mm ⁴			mber C20019 with 2 rows of Bridging le·l _x 450 MPa·G450·4.51 x 10 ⁶ mm ⁴ Span 3784 mm oads 2 Point Loads at 1656 & 3062 mm I Req 3.004 kN/m I Req 2.239 kN/m acity 6.660 kN/m·2 rows of Bridging·Span @			
⊘M Hip Ra	25.700 kNm · 100x Source: OneSteel® D	OuraGal [®] Hollow Sections	Gov Span · Load Width Capacity · Inward Req Outward Req	1646 mm · 1750 mm 1.446 kN/m	G Concentrate Capacity · Inw Outw	ov Span ed Loads vard Req vard Req	3784 mm 2 Point Load 3.004 kN/m 2.239 kN/m	ls at 1656 &	≨ 3062 mm
Details · Member Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Outward Req	25.700 kNm · 100x Source: OneSteel® D fter · HR · Calculation C15012 500 MPa · G500 · 1 2103 mm · 1750 m 1.446 kN/m 1.078 kN/m 6.400 kN/m · min(In	ons L.29 x 10 ⁶ mm ⁴ nm · Gov = Governing hward, Outward)	Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection	1646 mm · 1750 mm 1.446 kN/m 1.078 kN/m 8.460 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 0.490 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 841 · L _{ez} 1646 0.365 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 1646 · L _{ez} 1646 0.1 mm · 5.5 Permitted	G Concentrate Capacity · Inw Outw Member G ØM · Inw Outw	iov Span ed Loads vard Req vard Req Capacity vard Req vard Req ØM	3784 mm 2 Point Load 3.004 kN/m 2.239 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.377 kNm 4.008 kNm 14.823 kNm	s at 1656 & 2 rows of Bi ht® & Stram	ridging · Span @ iit® Span Tables Ley 1656 · Lez 1656
Details · Member Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Outward Req	25.700 kNm · 100x Source: OneSteel® D fter · HR · Calculation C15012 500 MPa · G500 · 1 2103 mm · 1750 m 1.446 kN/m 1.078 kN/m 6.400 kN/m · min(In Source: Lysaght® & S	ons 1.29 x 10 ⁶ mm ⁴ nm · Gov = Governing	Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection	1646 mm · 1750 mm 1.446 kN/m 1.078 kN/m 8.460 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 0.490 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 841 · L _{ez} 1646 0.365 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 1646 · L _{ez} 1646 0.1 mm · 5.5 Permitted	G Concentrate Capacity · Inw Outw Member G	iov Span ed Loads vard Req vard Req Capacity vard Req vard Req ØM flection	3784 mm 2 Point Load 3.004 kN/m 2.239 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.377 kNm 4.008 kNm 14.823 kNm 0.8 mm · 12.6	s at 1656 & 2 rows of Binht® & Stram • L _{ex} 3784 • I mm Permit	ridging · Span @ rit® Span Tables Ley 1656 · Lez 1656 tted
Details · Member Details · Member Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward OM	25.700 kNm · 100x Source: OneSteel® D fter · HR · Calculation C15012 500 MPa · G500 · 1 2103 mm · 1750 m 1.446 kN/m 1.078 kN/m 6.400 kN/m · min(In Source: Lysaght® & S 0.799 kNm 5.310 kNm · L _{ex} 210 0.596 kNm	cons 1.29 x 10 ⁶ mm ⁴ 1m · Gov = Governing 1.29 x 10 ⁶ mm ⁴ 1m · Gov = Tables 1.29 x 10 ⁶ mm ⁴ 1m · Gov = Harris Span Tables 1.29 x 10 ⁶ mm ⁴ 1.20 x	Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection Govern Dutch End Purlin Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req	1646 mm · 1750 mm 1.446 kN/m 1.078 kN/m 8.460 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 0.490 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 841 · L _{ez} 1646 0.365 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 1646 · L _{ez} 1646 0.1 mm · 5.5 Permitted ning Purlin Calculations TH6110 550 MPa · G550 · 0.16 x 106mm4 2630 mm · 841 mm	G Concentrate Capacity · Inw Outw Member G ØM · Inw Outw Def Fascia Details · Min Yield Stress · G G	ov Span ed Loads vard Req vard Req Capacity vard Req vard	3784 mm 2 Point Load 3.004 kN/m 2.239 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.377 kNm 4.008 kNm 14.823 kNm 0.8 mm · 12.0 alculations for C20019 with 450 MPa · G 3820 mm	2 rows of British & Stram Lex 3784 · L 6 mm Permit 10 FB2.1 & 3 rows of British 1450 · 4.51 x	ridging · Span @ ridging · Span @ rit® Span Tables Ley 1656 · Lez 1656 tted FB2.2 idging c 10 ⁶ mm ⁴
Mip Ra Details · Member Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward ØM Outward ØM Deflection	25.700 kNm · 100x Source: OneSteel® D fter · HR · Calculation C15012 500 MPa · G500 · 1 2103 mm · 1750 m 1.446 kN/m 1.078 kN/m 6.400 kN/m · min(In Source: Lysaght® & S 0.799 kNm 5.310 kNm · L _{ex} 210 0.596 kNm 5.310 kNm · L _{ex} 210	ons 1.29 x 10 ⁶ mm ⁴ mw·Gov = Governing bward, Outward) Stramit® Span Tables 33 · L _{ey} 841 · L _{ez} 2103 cermitted	Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection Govern Dutch End Purlin Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Inward Capacity	1646 mm · 1750 mm 1.446 kN/m 1.078 kN/m 8.460 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 0.490 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 841 · L _{ez} 1646 0.365 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 1646 · L _{ez} 1646 0.1 mm · 5.5 Permitted ning Purlin Calculations TH6110 550 MPa · G550 · 0.16 x 10 ⁶ mm ⁴ 2630 mm · 841 mm 1.042 kN/m 1.550 kN/m · Single Span @ 3000mm Source: Lysaght® & Stramit® Span Tables	G Concentrate Capacity · Inw Outw Member G ØM · Inw Outw Def Fascia Details · Min Yield Stress · G G	iov Span ed Loads vard Req vard Req Capacity vard Req ØM flection a Beam C Member Grade · I _x iov Span ed Loads	3784 mm 2 Point Load 3.004 kN/m 2.239 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.377 kNm 4.008 kNm 14.823 kNm 0.8 mm · 12.0 alculations for C20019 with 450 MPa · G 3820 mm 3 Point Load	2 rows of British & Stram Lex 3784 · L 6 mm Permit 10 FB2.1 & 3 rows of British 1450 · 4.51 x	ridging · Span @ ridging · Span @ it® Span Tables Ley 1656 · Lez 1656 tted FB2.2 idging
Hip Ra Details · Member Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity	25.700 kNm · 100x Source: OneSteel® D fter · HR · Calculation C15012 500 MPa · G500 · 1 2103 mm · 1750 m 1.446 kN/m 1.078 kN/m 6.400 kN/m · min(In Source: Lysaght® & S 0.799 kNm 5.310 kNm · L _{ex} 210 0.596 kNm 5.310 kNm · L _{ex} 210 0.2 mm · 7.0 mm Per E Strut · RLS · Calculation C15012 500 MPa · G500 · 1 1390 mm	DuraGal® Hollow Sections Ons L.29 x 10 ⁶ mm ⁴ Im · Gov = Governing Distramit® Span Tables	Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection Govern Dutch End Purlin Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Inward Capacity Outward Req	1646 mm · 1750 mm 1.446 kN/m 1.078 kN/m 8.460 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 0.490 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 841 · L _{ez} 1646 0.365 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 1646 · L _{ez} 1646 0.1 mm · 5.5 Permitted ning Purlin Calculations TH6110 550 MPa · G550 · 0.16 x 10 ⁶ mm ⁴ 2630 mm · 841 mm 1.042 kN/m 1.550 kN/m · Single Span @ 3000mm	G Concentrate Capacity · Inw Outw Member (ØM · Inw Outw Det Fascii Details · Min Yield Stress · G Concentrate Capacity · Inw Outw Member (Member (ov Spaned Loads vard Requard R	3784 mm 2 Point Load 3.004 kN/m 2.239 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.377 kNm 4.008 kNm 14.823 kNm 0.8 mm · 12.4 alculations f C20019 with 450 MPa · G 3820 mm 3 Point Load 3.102 kN/m 2.314 kN/m 6.660 kN/m 3900 mm	Lex 3784 · L 6 mm Permit or FB2.1 & 3 rows of Brids at 504, 15	ridging · Span @ ridging · Span @ rit® Span Tables Ley 1656 · Lez 1656 tted FB2.2 idging c 10 ⁶ mm ⁴
Hip Ra Details · Member Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity	25.700 kNm · 100x Source: OneSteel® D fter · HR · Calculation C15012 500 MPa · G500 · 1 2103 mm · 1750 m 1.446 kN/m 1.078 kN/m 6.400 kN/m · min(In Source: Lysaght® & S 0.799 kNm 5.310 kNm · L _{ex} 210 0.596 kNm 5.310 kNm · L _{ex} 210 0.2 mm · 7.0 mm Per E Strut · RLS · Calculation C15012 500 MPa · G500 · 1	DuraGal® Hollow Sections ons 1.29 x 10 ⁶ mm ⁴ Im · Gov = Governing Duward, Outward) Stramit® Span Tables 03 · Ley 841 · Lez 2103 03 · Ley 2103 · Lez 2103 Darmitted Ilations 1.29 x 10 ⁶ mm ⁴ PROJECT: Example 7000 x 120	Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection Govern Dutch End Purlin Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Inward Capacity Outward Req	1646 mm · 1750 mm 1.446 kN/m 1.078 kN/m 8.460 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 0.490 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 841 · L _{ez} 1646 0.365 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 1646 · L _{ez} 1646 0.1 mm · 5.5 Permitted ning Purlin Calculations TH6110 550 MPa · G550 · 0.16 x 106mm4 2630 mm · 841 mm 1.042 kN/m 1.550 kN/m · Single Span @ 3000mm Source: Lysaght® & Stramit® Span Tables 0.777 kN/m 1.380 kN/m · Single Span @ 3000 mm · Minimum Support Thickness of 1.2 mm BMT 2 x No. 12 Tek Screws	G Concentrate Capacity · Inw Outw Member G ØM · Inw Outw Def Fascia Details · Min Yield Stress · G Concentrate Capacity · Inw Outw Member G ØM · Inw ROJECT NUMBER: 6457	iov Span ed Loads vard Req vard Req vard Req vard Req om flection a Beam C Member Grade · I _x iov Span ed Loads vard Req capacity	3784 mm 2 Point Load 3.004 kN/m 2.239 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.377 kNm 4.008 kNm 14.823 kNm 0.8 mm · 12.0 alculations for C20019 with 450 MPa · G 3820 mm 3 Point Load 3.102 kN/m 2.314 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.658 kNm 3 Solutions. The Intant of the Sole Propele is Prohibited with	s at 1656 & 2 rows of Bitht® & Stram • L _{ex} 3784 • L 6 mm Permitor FB2.1 & 3 rows of Britht® 450 • 4.51 x Is at 504, 1: • 3 rows of Britht® & Stram formation containerty of dm3 Soluthout written appr	ridging · Span @ ridging · Span @ lit® Span Tables Ley 1656 · Lez 1656 tted FB2.2 idging (106mm4 910 & 3316 mm ridging · Span @ lit® Span Tables med herein is Proprietary, itons. Reproduction in part roval. Unless specified
Hip Ra Details · Member Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity	25.700 kNm · 100x Source: OneSteel® D fter · HR · Calculation C15012 500 MPa · G500 · 1 2103 mm · 1750 m 1.446 kN/m 1.078 kN/m 6.400 kN/m · min(In Source: Lysaght® & S 0.799 kNm 5.310 kNm · L _{ex} 210 0.596 kNm 5.310 kNm · L _{ex} 210 0.2 mm · 7.0 mm Pe a Strut · RLS · Calcul C15012 500 MPa · G500 · 1 1390 mm 1.485 kN	ons 1.29 x 10 ⁶ mm ⁴ am · Gov = Governing nward, Outward) Stramit® Span Tables 1.3 · L _{ey} 841 · L _{ez} 2103 1.3 · L _{ey} 2103 · L _{ez} 2103 armitted lations 1.29 x 10 ⁶ mm ⁴	Gov Span · Load Width Capacity · Inward Req Outward Req Member Capacity ØM · Inward Req Inward ØM Outward Req Outward ØM Deflection Govern Dutch End Purlin Min Yield Stress · Grade · I _x Gov Span · Load Width Capacity · Inward Req Inward Capacity Outward Req Outward Capacity	1646 mm · 1750 mm 1.446 kN/m 1.078 kN/m 8.460 kN/m · min(Inward, Outward) Source: Lysaght® & Stramit® Span Tables 0.490 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 841 · L _{ez} 1646 0.365 kNm 5.310 kNm · L _{ex} 1646 · L _{ey} 1646 · L _{ez} 1646 0.1 mm · 5.5 Permitted ning Purlin Calculations TH6110 550 MPa · G550 · 0.16 x 106mm4 2630 mm · 841 mm 1.042 kN/m 1.550 kN/m · Single Span @ 3000mm Source: Lysaght® & Stramit® Span Tables 0.777 kN/m 1.380 kN/m · Single Span @ 3000 mm · Minimum Support Thickness of 1.2 mm BMT 2 x No. 12 Tek Screws	G Concentrate Capacity · Inw Outw Member (ØM · Inw Outw Det Fascia Details · Min Yield Stress · G G Concentrate Capacity · Inw Outw Member (ØM · Inw ROJECT NUMBER:	iov Span ed Loads vard Req vard Req vard Req vard Req om flection a Beam C Member Grade · I _x iov Span ed Loads vard Req capacity	3784 mm 2 Point Load 3.004 kN/m 2.239 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.377 kNm 4.008 kNm 14.823 kNm 0.8 mm · 12.0 alculations for C20019 with 450 MPa · G 3820 mm 3 Point Load 3.102 kN/m 2.314 kN/m 6.660 kN/m 3900 mm Source: Lysag 5.658 kNm 3 Solutions. The Intant of the Sole Propele is Prohibited with	s at 1656 & 2 rows of Britht® & Stram • L _{ex} 3784 • L 6 mm Permit or FB2.1 & 3 rows of Britht® 450 • 4.51 x Is at 504, 1: • 3 rows of Britht® & Stram formation contain rety of dm3 Solut hout written appr millimetres & dra	ridging · Span @ ridging · Span @ lit® Span Tables Ley 1656 · Lez 1656 tted FB2.2 idging (106mm4 910 & 3316 mm ridging · Span @ lit® Span Tables med herein is Proprietary, tions. Reproduction in part towal. Unless specified awings are not to scale.

Outward Reg 4.22 kNm ØM **14.823 kNm** · L_{ex} 3820 · L_{ev} 1406 · L_{ez} 1406 **Deflection 1.1 mm** 12.7 mm Permitted Fascia Beam Calculations for FB3.1 & FB3.2 **Details** · Member **C20019** with 2 rows of Bridging Min Yield Stress · Grade · I_x 450 MPa · G450 · 4.51 x 10⁶mm⁴ Gov Span 3784 mm Concentrated Loads 2 Point Loads at 722 & 2128 mm Capacity · Inward Req 4.477 kN/m Outward Reg 3.337 kN/m Member Capacity 6.660 kN/m ⋅ 2 rows of Bridging ⋅ Span @ 3900 mm Source: Lysaght[®] & Stramit[®] Span Tables ØM · Inward Reg 8.013 kNm Outward Reg 5.973 kNm ØM **14.823 kNm** · L_{ex} 3784 · L_{ev} 1656 · L_{ez} 1656 **Deflection 0.6 mm** · 12.6 mm Permitted **Cladding Details** Product **BlueScope**[®] **Colorbond**[®] Min Yield Stress · Grade 550 MPa · G550 Profile CustomOrb®

> | End Posts Bored Piers | ØM · Mu | 11.03 kNm | Uplift · Vu | 4.589 kN

BMT **0.42 mm**

Dimensions 350 mm diameter x 1100 mm deep

Internal Posts Bored Piers

Shear · H., 4.596 kN

Dimensions 450 mm diameter x 1300 mm deep

Connection Calculations

Connection Type
Components
Capacity Rea
Capa

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type Common Rafter to Fascia
Components 2 x M12 4.6/S Bolts

Capacity Req Capacity Solution Capacity Capacity Solution Capacity Solution Capacity Capacity Req Capacity Re

Connection Type Internal Common Rafter to Fascia

Components 2 x M12 4.6/S Bolts

Capacity Req 3.878 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type L/R Fascia Beam to Post
Components 2 x M12 4.6/S Bolts

Capacity Reg 1.346 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type FB1.1 & FB1.2 to Post Components 2 x M12 4.6/S Bolts

Capacity Req 5.684 kN

Capacity 30.2 kN · 2 x M12 4.6/S Shear Capacity

Connection Type FB2.1 & FB2.2 to Post Components 2 x M12 4.6/5 Bolts

Capacity Req 5.925 kN

Capacity **30.2 kN** · 2 x M12 4.6/S Shear Capacity

Connection Type FB3.1 & FB3.2 to Post 2 x M12 4.6/5 Bolts

Capacity Req 8.470 kN

Capacity **30.2 kN** · 2 x M12 4.6/S Shear Capacity

1 x M12 4.6/S Bolt Web Bearing Capacity

 $\begin{array}{lll} \textbf{24.883 kN} \cdot 1.5 \text{ mm Gauge C-Section} \\ \textbf{31.519 kN} \cdot 1.9 \text{ mm Gauge C-Section} \\ \textbf{39.813 kN} \cdot 2.4 \text{ mm Gauge C-Section} \\ \end{array}$

Plate Bearing Capacity

49.766 kN · 3.0 mm Gauge Plate

1 x M16 8.8/S Bolt Web Bearing Capacity

53.084 kN · 2.4 mm Gauge C-Section **66.355 kN** · 3.0 mm Gauge C-Section

Plate Bearing Capacity

66.355 kN · 3.0 mm Gauge Plate

Plates Tear Out Capacities

Minimum · Top Bolt 51.840 kN Minimum · Bottom Bolt 77.760 kN

Your Logo Here						

	Example 7000 x 12000 x 2400 Region B1 Dutch Gable Carport	PROJECT NUMBER: 6457	Confidential and	d the Sole Prope	rty of dm3 Soluti	led herein is Proprietary, ions. Reproduction in part oval. Unless specified	ì
	CLIENT:	DRAWN BY:	otherwise, dimensions are in millimetres & drawings are not to scale.				
_	Example Customer	dm3 Solutions	Powered by dm3Solutions.com				ì
	ADDRESS:	DRAWN DATE:	ISSUE:	SIZE:	SCALE:	DRAWING NUMBER	
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